




CLUBHOUSE & APARTMENT  
FOR  
NORTH BEACH MINI GOLF

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**JOHN P. WANSKUS**  
**RA NCARB CGC CCC**  
**LICENSES**  
☐ CT ~ #ARI 0013326  
☐ FL ~ #AR93250  
☐ IL ~ #001-014842  
☐ MA ~ #32317  
☐ NCARB ~ #67566  
☒ NJ ~ #21A101169200  
☐ NY ~ #03-034472  
☐ NC ~ #014537  
☐ SC ~ #AR9931  
☐ TX ~ #21153  
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DRAWING TITLE  
**COVER  
SHEET**

NO.	REV.	DATE
OWNER / LOCATION NORTH BEACH MINI GOLF 120 EUCLID AVENUE ATLANTIC CITY, NJ		
DRAWN BY: JPW CHECKED BY: JPW PROJECT # 22-2507.178 DATE: 25 JULY 2022		

SHEET NO.  
**T-1**  
1 OF 20

DRAWING INDEX

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120 Euclid Ave, Atlantic City, NJ 08401  
Location: 39.367752, -74.411289

NORTH BEACH MINI GOLF



KEY MAP

SCALE: 1" = 1000'



JOHN P. WANSKUS  
RA NCARB CGC CCC

- LICENSES
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DWG INDEX  
KEY MAP

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DRAWN BY: JPW CHECKED BY: JPW PROJECT # 22-2507.178 DATE: 25 JULY 2022		

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C-1  
2 OF 20

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- THIS PLAN WAS PREPARED USING THE FOLLOWING REFERENCES:  
- EXISTING CONDITIONS PLAN BY ARTHUR W. FORZIO CO. & ASSOC.  
DATED 12-22-2020
- THIS SITE IS LOCATED WITHIN AN "AE", "COASTAL A" AND "VE" ZONE
- THE FEMA VE ZONE, ELEV 14.00 CONTROLS
- FEMA FLOOD ELEV = 14.00 (NAVD 88 DATUM)

PROP. (2) 3"x10" 45° DIAGONALS EAST/WEST  
IN THIS (4) PILE GROUP PLUS (2) 3"x10" 45°  
NORTH/SOUTH IN THIS SAME PILE GROUP  
BOLTED BOTH ENDS W/ 1" DIA. HOT DIP  
GALV. BOLTS, NUTS AND (2) PLATE WASHERS

PROP. 3"x8"  
P.T. GIRDER  
ON (20) 12" P.T.  
BUTT PILES  
NOTCHED (TYP)

TRIM JOIST AS REQUIRED  
TO PREVENT LIP IN  
DECKING

0.00 PROPOSED SPOT  
ELEVATION (TYP)

PROP. 2"x10"  
P.T. JOISTS  
NOTCH 3"x8"  
GIRDER  
INTO PILE (TYP)

PROP. 13'-11"± LONG  
P.T. 8" X 12" TIMBER  
PILE CAP (TYP)

PROPOSED DBL  
P.T. 3"x12" JOIST  
12'-7" LONG

PROP. 12'-0"± LONG  
P.T. 8" X 12" TIMBER  
PILE CAP (TYP)

PROP. 35'± LONG P.T.  
MIN. 12" DIA. BUTT  
PILES COMPLIANT  
TO ASTM D55 (TYP)

PROP. P.T. DBL  
3"x10" W/ P.T. 2"x10"  
SPACER AND 2" IPE  
DIVIDER BOARD W/  
SCREWS 24" OC.  
(3) ROWS MAIN DECK

PROP. 2"x4" IPE  
DECKING ON DIAGONAL  
CUT FROM 16' LONG  
BOARDS. NO SPICING  
PERMITTED BETWEEN  
DIVIDER BOARDS

PROP. P.T. 3"x10" JOIST  
11' LONG (TYP) 16" OC

PROP. PAINTED STEEL  
BEAM (2) PLACES  
o W10X54, OR  
o W12X40, OR  
o W14X34 WITH SUPPORT  
PILE TRIMMED TO CAUSE  
TOP OF STEEL TO BE  
FLUSH W/ 8"x12" PILE  
CAP TOPS.  
NORTH BEAM 23'-9" LONG  
SOUTH BEAM 23'-1" LONG

PROP. DBL  
3"x10" JOIST

PROP. PAINTED STEEL L6X6X1  
FROM 8' LONG STOCK W/ 8"  
LONG, TAB BOTH ENDS W/ (2)  
GALV. 3" A325 BOLTS SPACED 5"  
APART FROM EACH OTHER  
CENTERED ON TABS BOTH ENDS,  
BOLTED TO WEBS (TYP 2 PLACES)

PROP. PAINTED STEEL L6X6X1  
FROM 12'-8" LONG STOCK W/ 8"  
LONG, TAB 45° BOTH ENDS W/ (2)  
GALV. 3" A325 BOLTS SPACED 5"  
APART FROM EACH OTHER  
CENTERED ON TABS BOTH ENDS,  
BOLTED TO WEBS (TYP 2 PLACES)

PROP. P.T. DBL  
3"x10" W/ P.T. 2"x10"  
SPACER AND 2" IPE  
DIVIDER BOARD W/  
SCREWS 24" OC.  
(3) ROWS ON BRIDGE  
DECK. TRIM EAST 3"x10"  
AROUND EXIST BOARDWALK  
RAIL AS NEEDED

STORM WATER  
CONTROL  
STRUCTURE

EXISTING TIMBER  
BOARDWALK

PROP. 12'x24'  
PRE-FAB  
CLUBHOUSE  
BUILDING  
F.F. ELEV.=16.0'  
SKIRTING BELOW  
AT PERIMETER

(2) SETS OF  
(3) PILES WITH  
8"x12" GIRDER  
FOR CLUBHOUSE  
SUPPORT. BOTTOM  
OF GIRDER JOISTS  
ELEV.=15.00

PROP. SINGLE  
3"x12" JOIST  
12'-7" LONG

PROP. P.T. 3"x10" JOIST  
13'-4" LONG (TYP)

PROP. 2"x10" P.T.  
STAIR STRINGERS  
12" O.C.  
PROP. 3,500 PSI  
CONC FOOTING  
7' X 18" X 30"  
DEEP

PROP. DBL P.T. 3"x10"  
STRINGER SUPPORT

## SITE PLAN

SCALE: 1" = 10'

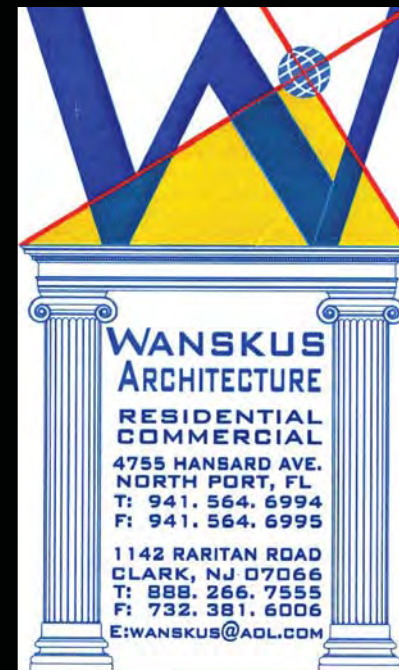
**Schaeffer Nassar Scheidegg**  
CONSULTING ENGINEERS, LLC  
Surveyors • Environmentalists • Planners  
1423 Canfield Boulevard • Maple Landing • New Jersey  
Telephone: (800) 983-7400 • Fax: (908) 381-0053  
Certificate of Authorization #240428 (08/00)



DECK PLAN FOR:  
120 EUCLID AVENUE, BLOCK 82, LOT 2  
CITY OF ATLANTIC CITY, ATLANTIC COUNTY, NEW JERSEY

DATE	BY	CHECKED	DATE	BY
3-3-21	B.G.	A.F.S.		
1-3-21	B.G.	A.F.S.		
1-3-21	B.G.	A.F.S.		
1-3-21	B.G.	A.F.S.		
1-3-21	B.G.	A.F.S.		
1-3-21	B.G.	A.F.S.		
1-3-21	B.G.	A.F.S.		
1-3-21	B.G.	A.F.S.		
1-3-21	B.G.	A.F.S.		

DATE: 3-3-21  
SCALE: 1/4"=1'-0"  
BY: B.G.  
CHECKED: A.F.S.  
SHEET NO. 1  
1 OF 3  
FILE# 21-149



## JOHN P. WANSKUS RA NCARB CGC CCC LICENSES

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## DRAWING TITLE SITE PLAN

NO.	REV.	DATE
1	1	3-3-21
2	1	3-3-21
3	1	3-3-21
4	1	3-3-21
5	1	3-3-21
6	1	3-3-21
7	1	3-3-21
8	1	3-3-21
9	1	3-3-21
10	1	3-3-21

DRAWN BY: JPW  
CHECKED BY: JPW  
PROJECT # 22-2507.178  
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SHEET NO. SP-1  
3 OF 20



BUILDING CODE DATA

1. USE GROUP: (B) BUSINESS - MINI GOLF CLUBHOUSE / (R-3) RESIDENTIAL
2. CONSTRUCTION CLASSIFICATION (STRUCTURE): TYPE VB (UNPROTECTED)
3. PROPOSED DESIGN: TWO-STORY WOOD FRAME BUILDING
4. BUILDING SIZE: 12'-0" W x 24'-0" L
5. BUILDING HEIGHT: 19'-4"
6. BUILDING AREA: FIRST FLOOR - 238 SF SECOND FLOOR - 238 SF  
TOTAL BUILDING AREA - 576 SF
7. DESIGN LOADS: ROOF: LIVE LOAD = 20 PSF (SNOW)  
DEAD LOAD = 13 PSF  
FIRST FLOOR: LIVE LOAD = 100 PSF  
DEAD LOAD = 10 PSF  
SECOND FLOOR: LIVE LOAD = 40 PSF  
DEAD LOAD = 10 PSF  
DECK: LIVE LOAD = 60 PSF  
DEAD LOAD = 13 PSF
8. DESIGN WIND SPEED: 125 MPH min. FOR "ULTIMATE DESIGN WIND SPEED"
9. WIND IMPORTANCE FACTOR: 1.00
10. WIND EXPOSURE: D ~ CATEGORY II
11. THE INTERNAL COEFFICIENT = 0

GENERAL NOTES

1. ALL MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE DRAWINGS AND SPECIFICATIONS LISTED.
2. ALL WORK SHALL BE INSTALLED SHALL BE OF THE HIGHEST QUALITY FOR EACH RESPECTIVE TRADE.
3. THE GENERAL CONTRACTOR SHALL COORDINATE ALL CONSTRUCTION SHOWN ON THE DRAWINGS. ANY QUESTIONS OR DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT.
4. ALL FOUNDATIONS ARE TO BE FORMED UPON SOIL WITH A MINIMUM SAFE UNIFORM BEARING CAPACITY OF 1,500 PSF.
5. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO PROVIDE THE UNIFORM BEARING CAPACITY UNDER THE FOUNDATION.
6. ANY SUBSTITUTIONS SHALL BE APPROVED IN WRITING BY OWNER AND ARCHITECT.
7. ALL CONCRETE CONSTRUCTION, WORKMANSHIP AND MATERIAL SHALL COMPLY WITH THE REQUIREMENTS OF ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS".
8. ALL CONCRETE SHALL BE FIBERMESH, TYPE I OR II COMPLYING WITH ASTM C150 AND SHALL HAVE COMPRESSIVE STRENGTH AND WATER/CEMENT RATIO (W/C) OF 3,000 PSI.
9. ALL REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60. BARS SHALL BE FREE OF COATINGS THAT WILL REDUCE CONCRETE BOND.
10. ALL CONCRETE REINFORCEMENT SHALL BE DETAILED, FABRICATED, LABELED, SUPPORTED AND SPACED IN FORMS AND SECURED IN PLACE IN ACCORDANCE WITH ACI "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT".
11. ALL BAR SPLICES, DOWELS AND CONCRETE COVERAGE SHALL MEET THE REQUIREMENTS OF ACI 318/318R "BUILDING CODE AND COMMENTARY FOR REINFORCED CONCRETE".
12. CONCRETE BEAMS AND SLABS SHALL BE FINISHED LEVEL AND TO THE ELEVATION SHOWN ON THE DRAWINGS AND SURVEY.
13. CALCIUM CHLORIDE SHALL NOT BE USED IN ANY FORM.
14. UNLESS OTHERWISE PERMITTED OR SPECIFIED, 3,000 PSI CONCRETE SHALL BE PRODUCED TO HAVE A SLUMP OF 6 INCHES +/- 1.5".
15. THE ARCHITECT RECOMMENDS THAT CONCRETE USED IN EXTERIOR SLABS HAVE A CORROSION INHIBITOR INCLUDED IN THE MIX CONTAINING CALCIUM NITRITE (30% SOLUTION @ 2.5 GAL/CY).
16. DRAWINGS ARE NOT TO BE SCALED FOR ANY PURPOSES. ANY DISCREPANCIES SHALL BE REPORTED TO THE ARCHITECT IMMEDIATELY FOR CLARIFICATION.
17. THESE PLANS HAVE BEEN PREPARED WITH THE 2018 INTERNATIONAL BUILDING CODE, N.J. EDITION, 2018 NATIONAL STANDARD PLUMBING CODE, N.J. EDITION, 2017 NATIONAL ELECTRICAL CODE, 2018 INTERNATIONAL MECHANICAL CODE, 2018 INTERNATIONAL FUEL GAS CODE, BARRIER FREE SUBCODE (ICC/ANSI A117.1-2009 (CHAPTER 11 OF IBC/2018 & NJAC 5:23-7).

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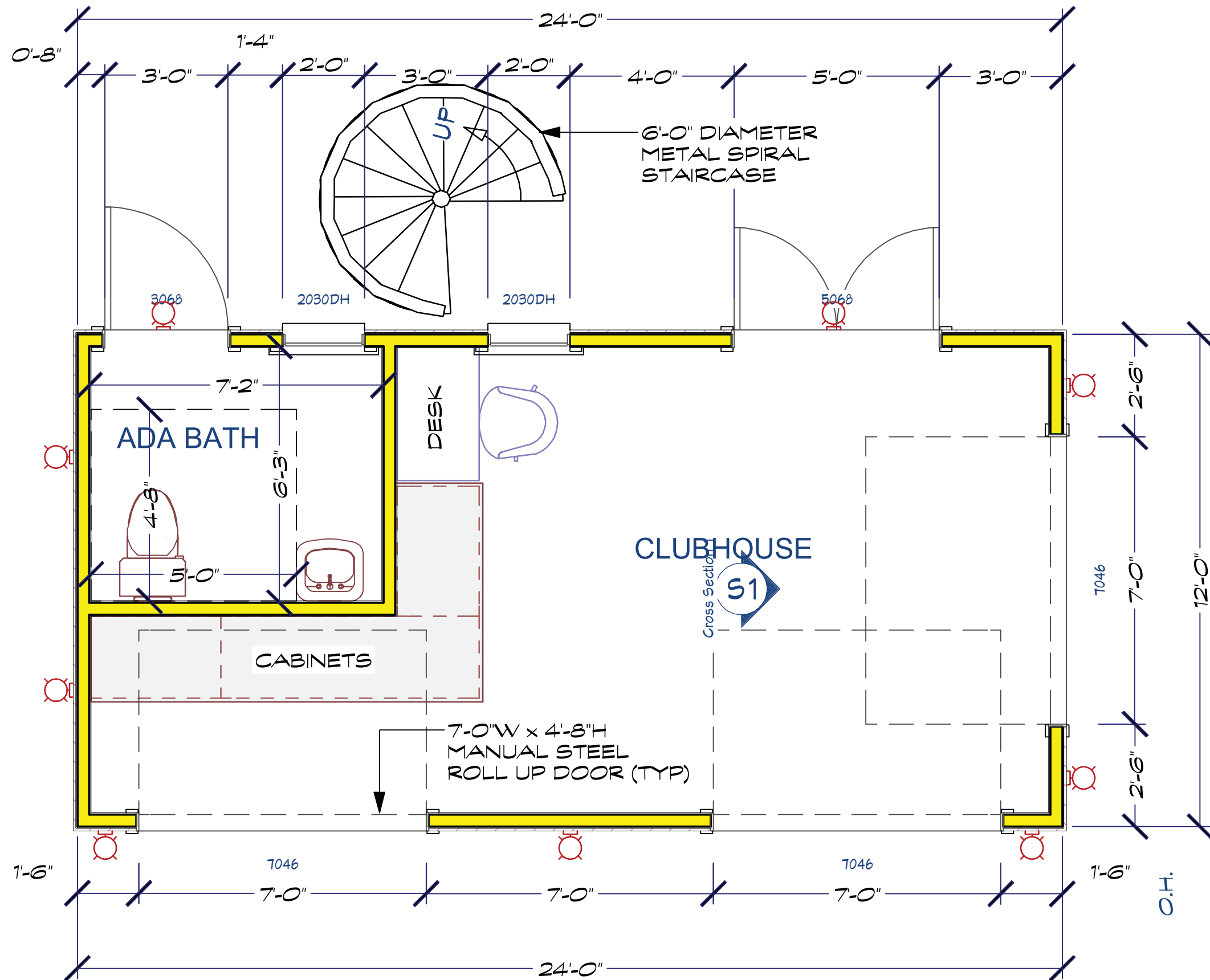
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**CODE & NOTES**

NO.	REV.	DATE
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DRAWN BY: JPW CHECKED BY: JPW PROJECT # 22-2507.178 DATE: 25 JULY 2022		
SHEET NO. <b>A-1</b> 4 OF 20		



**FIRST FLOOR PLAN**  
SCALE: 3/8" = 1'-0"

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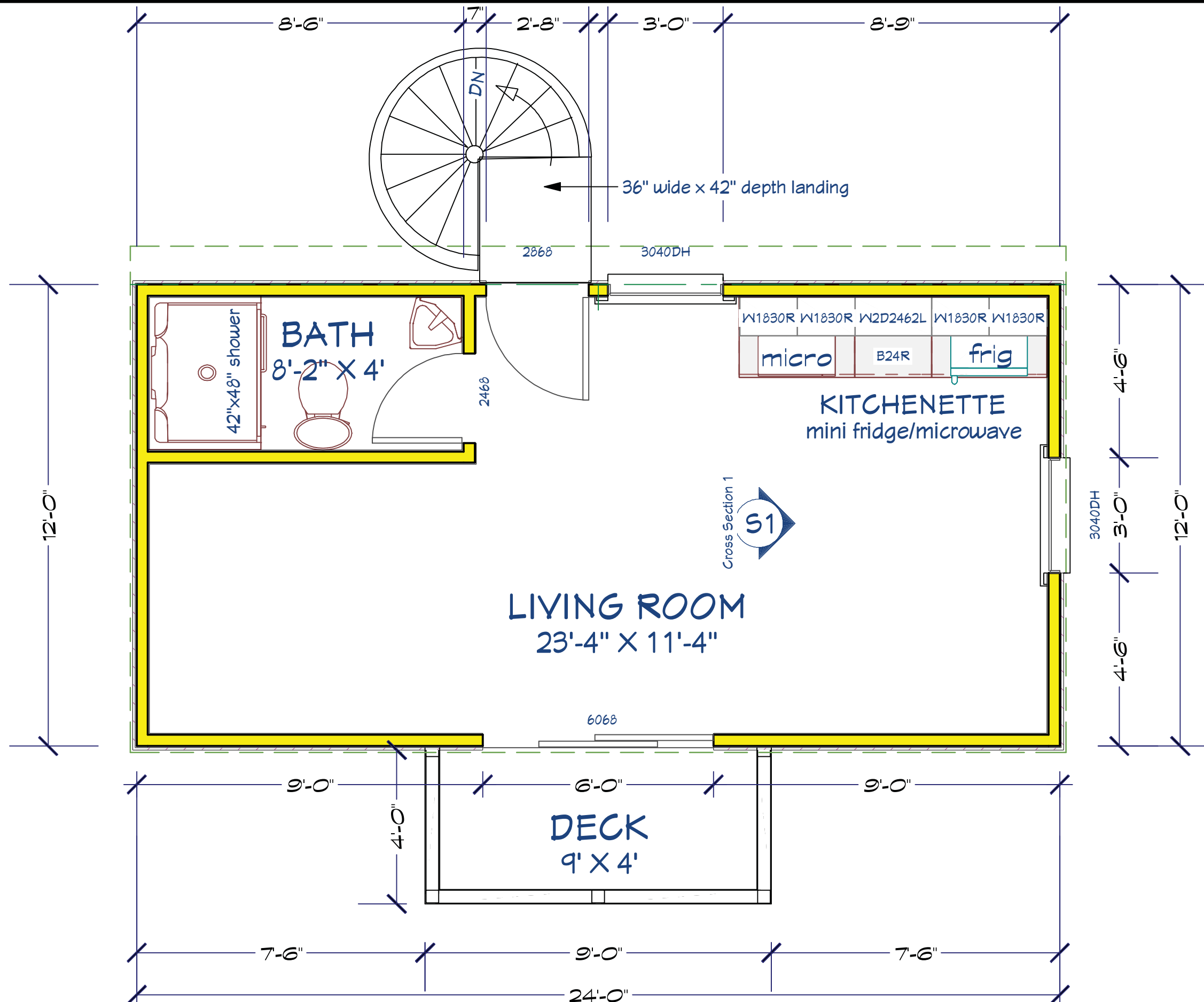
STATE OF NEW JERSEY  
JOHN PETER WANSKUS  
11692  
10/29/22  
REGISTERED ARCHITECT

DRAWING TITLE  
**FLOOR PLAN**  
**FIRST FLOOR**

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NORTH BEACH MINI GOLF		
120 EUCLID AVENUE		
ATLANTIC CITY, NJ		
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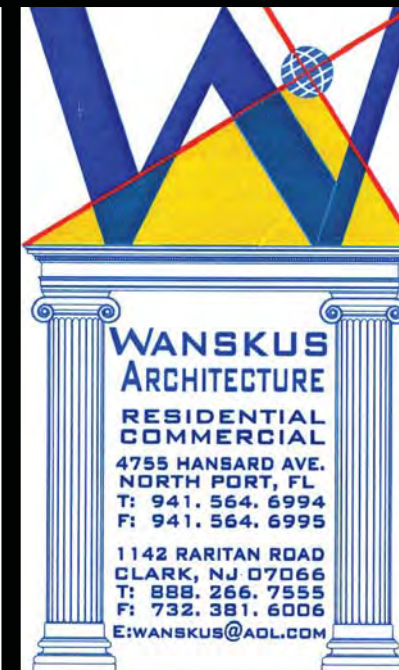
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## SECOND FLOOR PLAN

SCALE: 3/8" = 1'-0"



**JOHN P. WANSKUS**  
RA NCARB CGC CCC  
LICENSES

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DRAWING TITLE  
**FLOOR PLAN**  
**SECOND FLOOR**

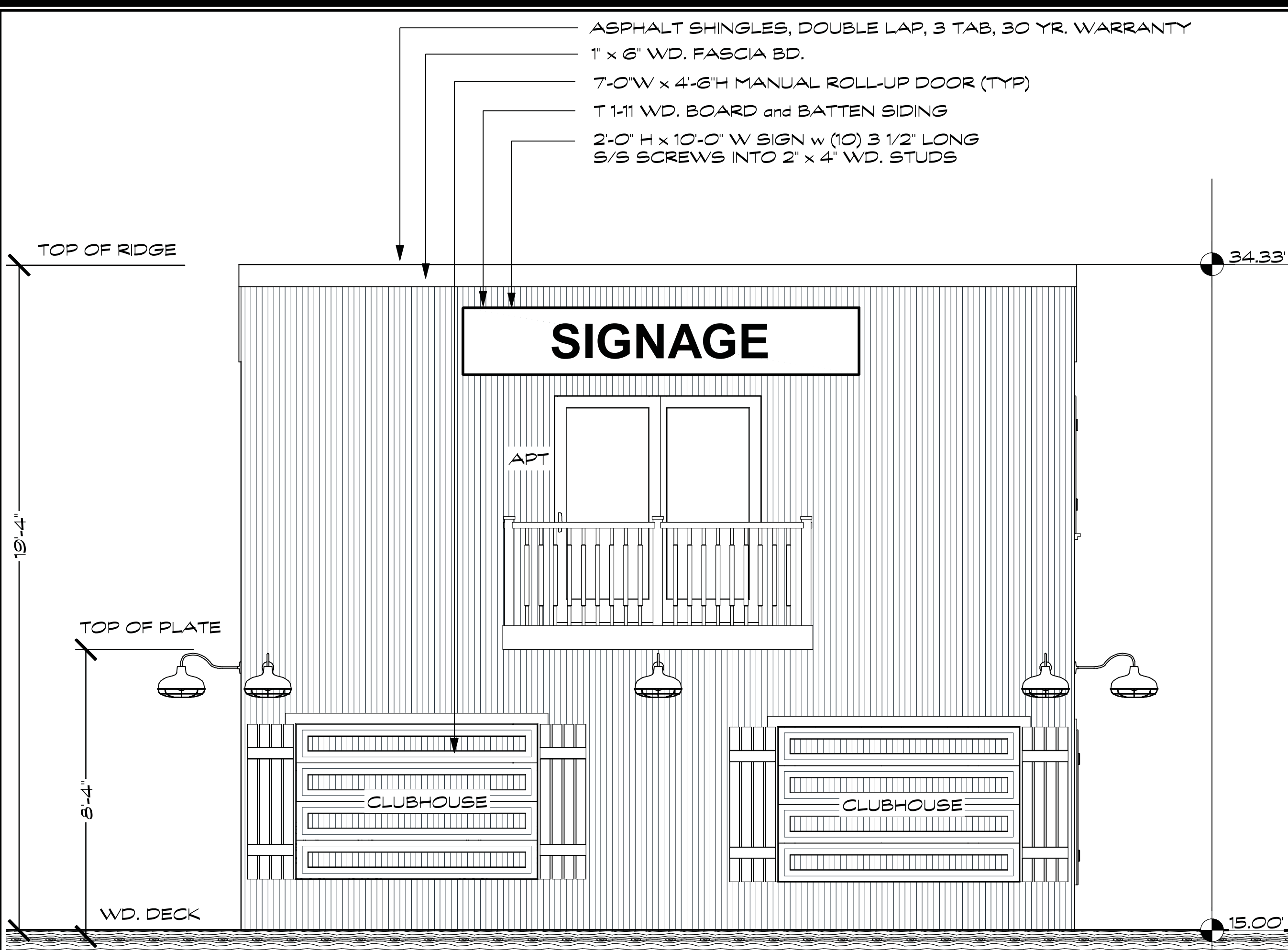
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ATLANTIC CITY, NJ		
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SHEET NO.

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- ASPHALT SHINGLES, DOUBLE LAP, 3 TAB, 30 YR. WARRANTY
- 1" x 6" WD. FASCIA BD.
- 7'-0"W x 4'-6"H MANUAL ROLL-UP DOOR (TYP)
- T 1-11 WD. BOARD and BATTEN SIDING
- 2'-0" H x 10'-0" W SIGN w (10) 3 1/2" LONG S/S SCREWS INTO 2" x 4" WD. STUDS

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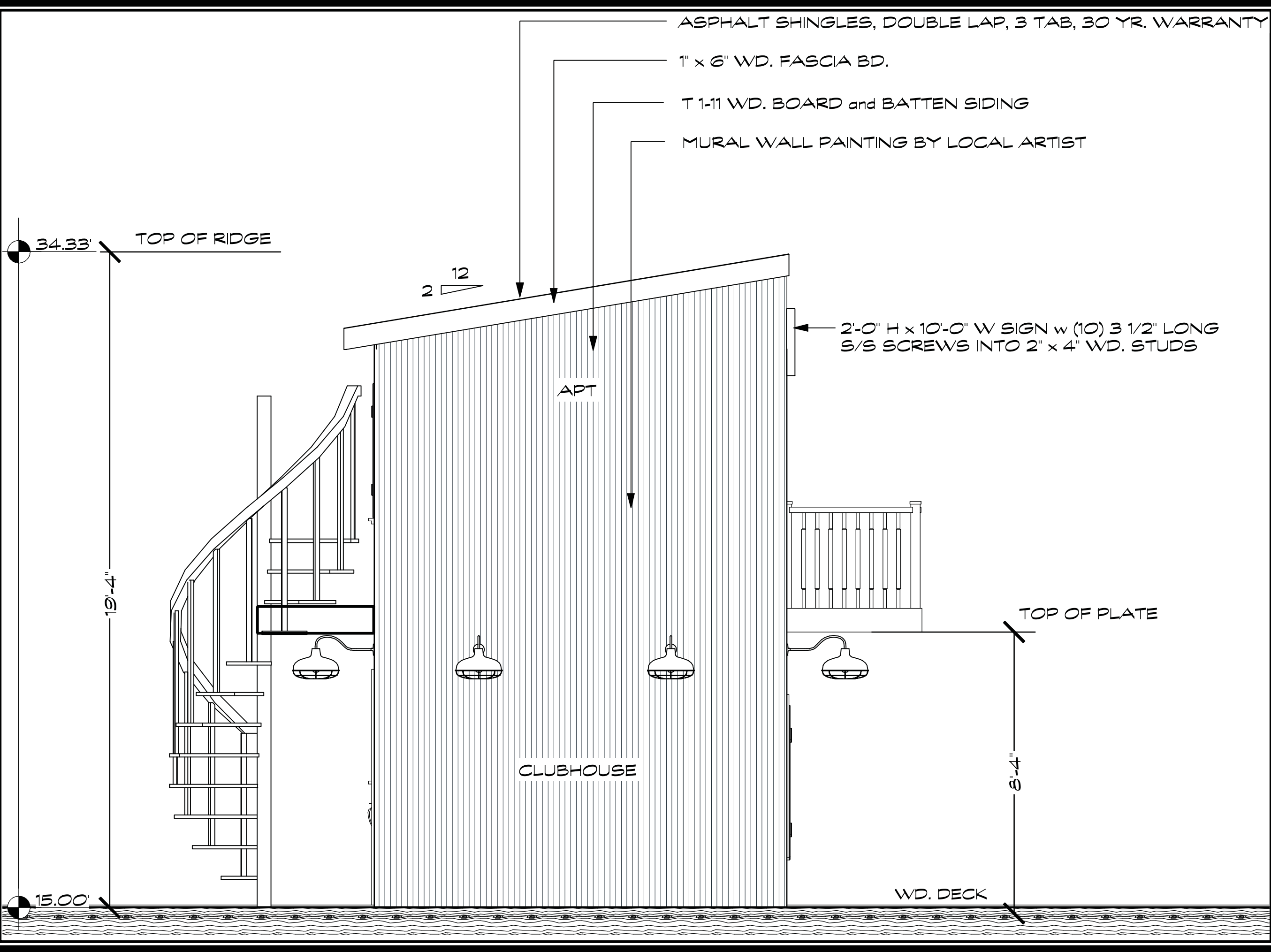


**DRAWING TITLE**  
**FRONT**  
**ELEVATION**

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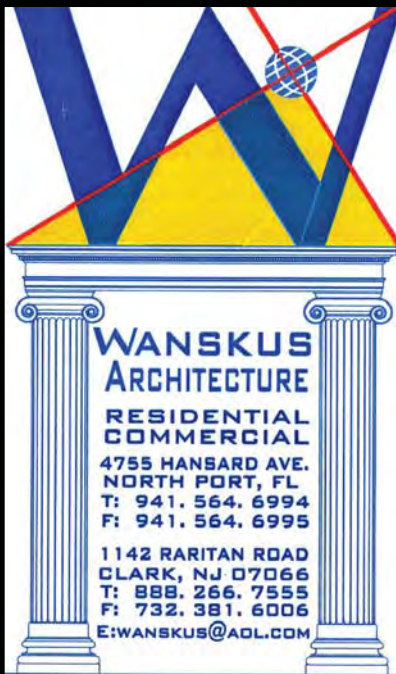
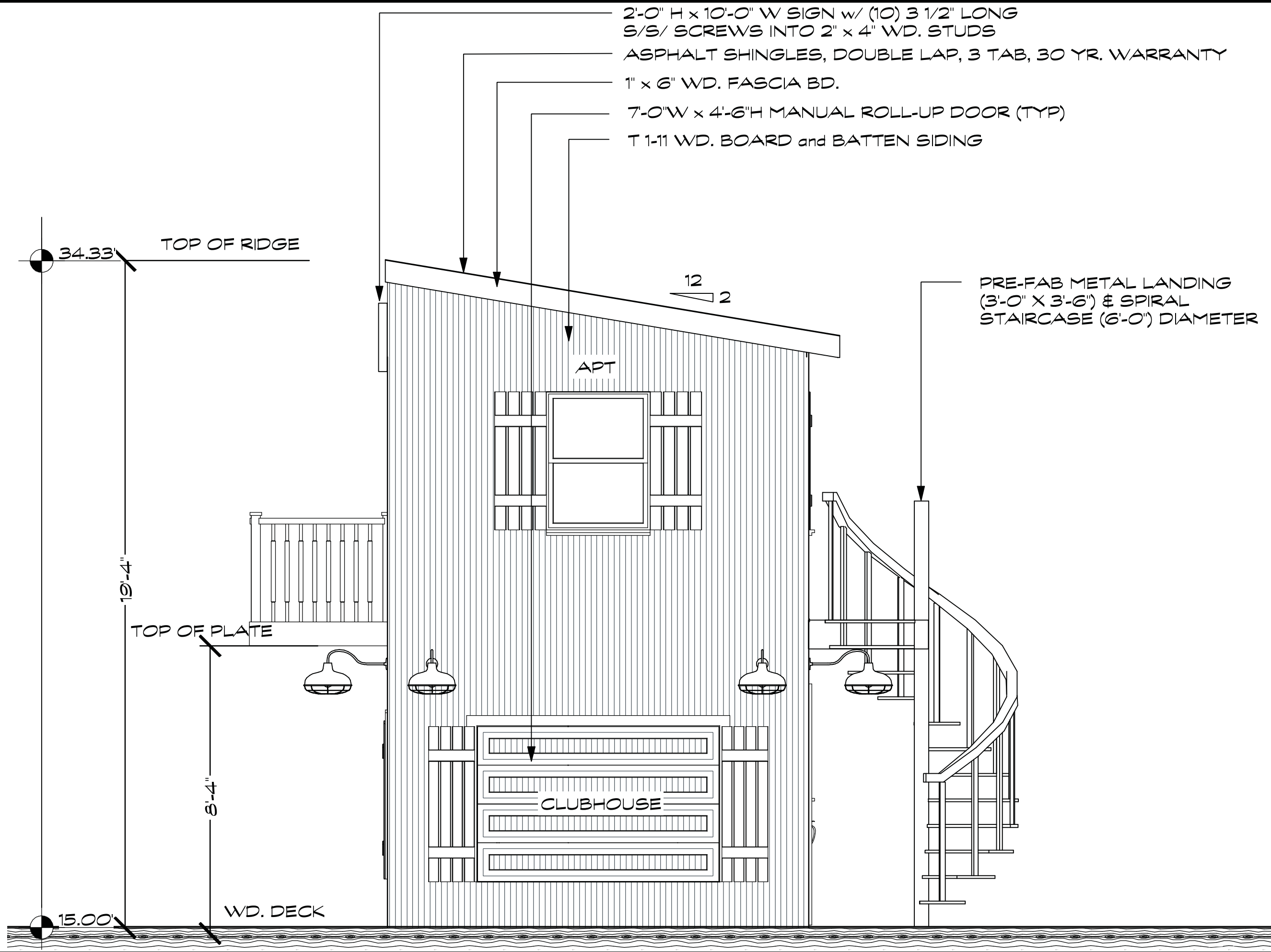
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DRAWING TITLE		
<b>LEFT SIDE ELEVATION</b>		
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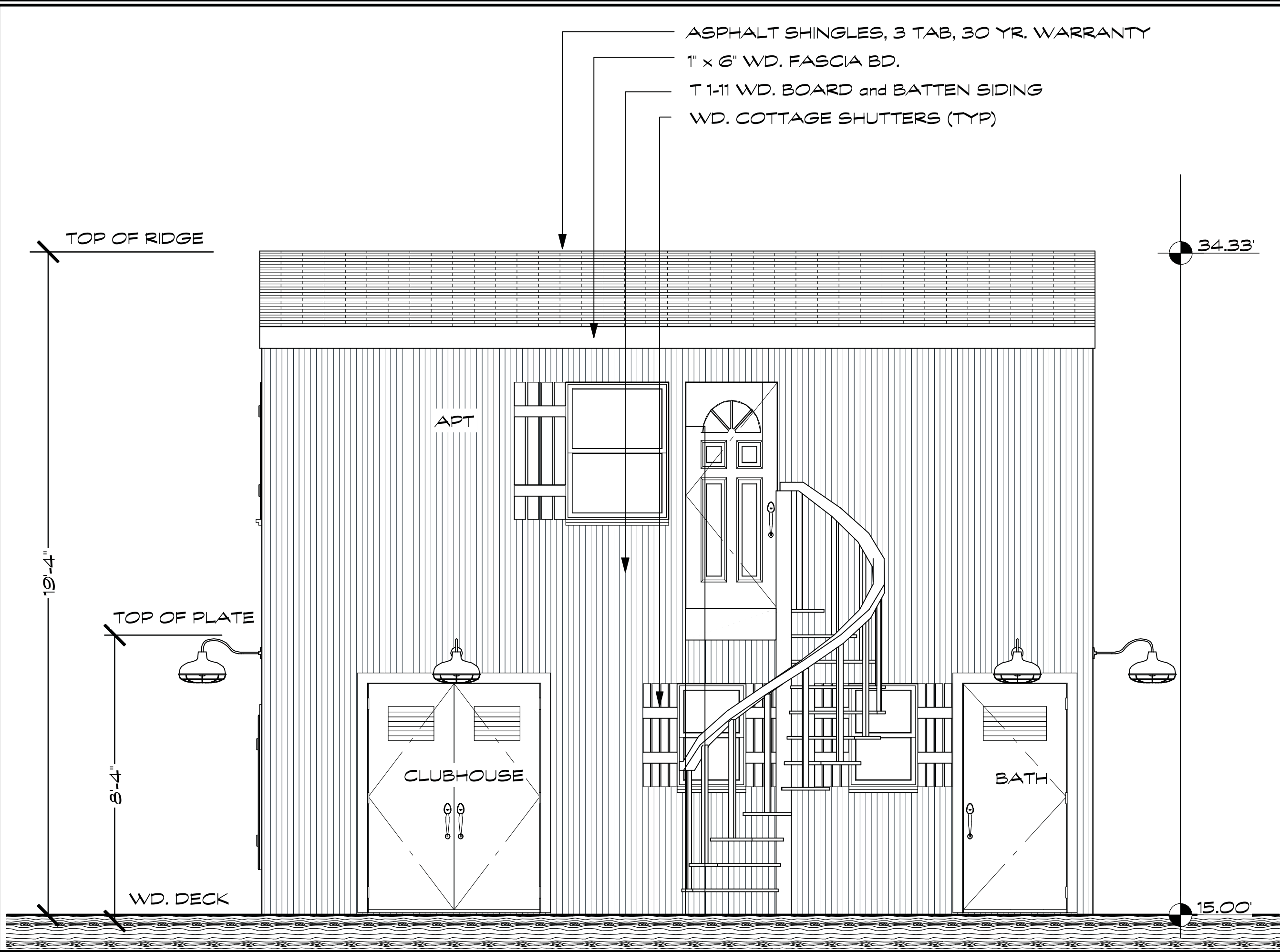
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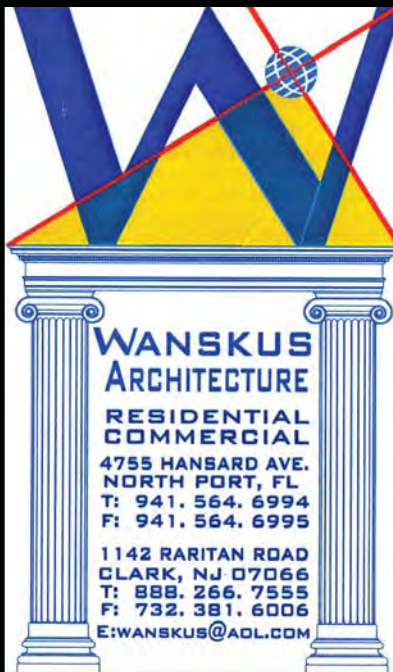
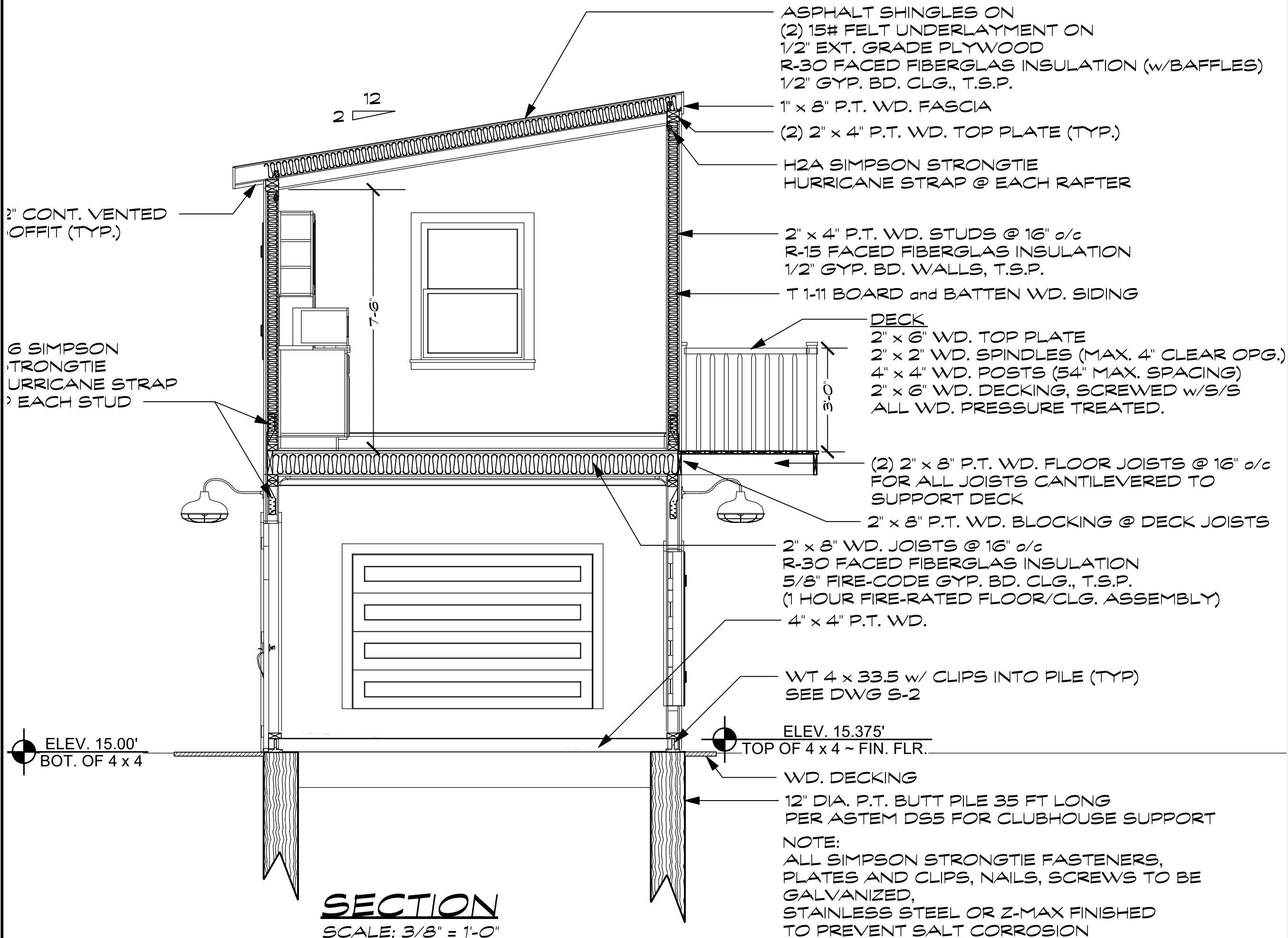
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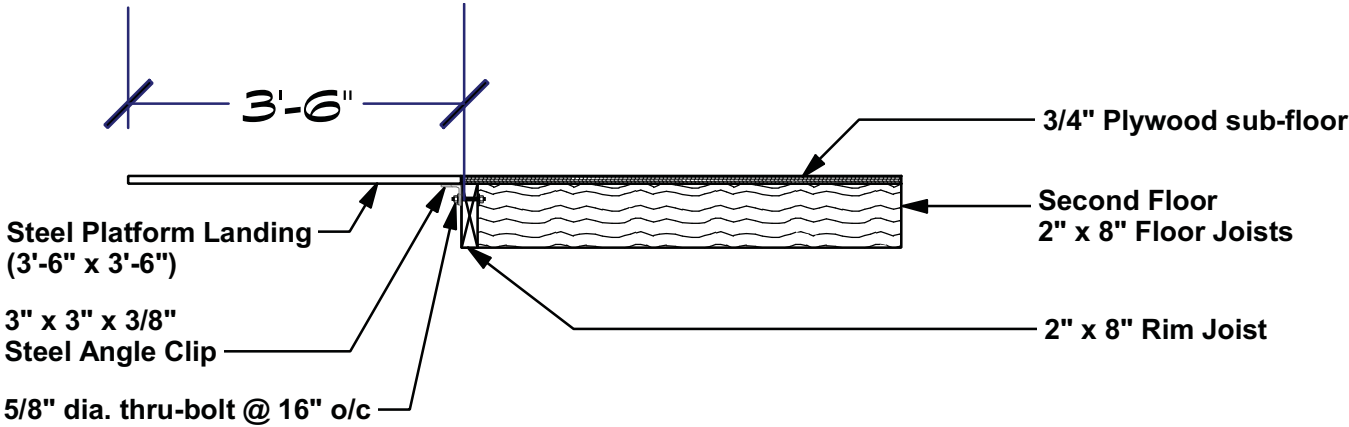
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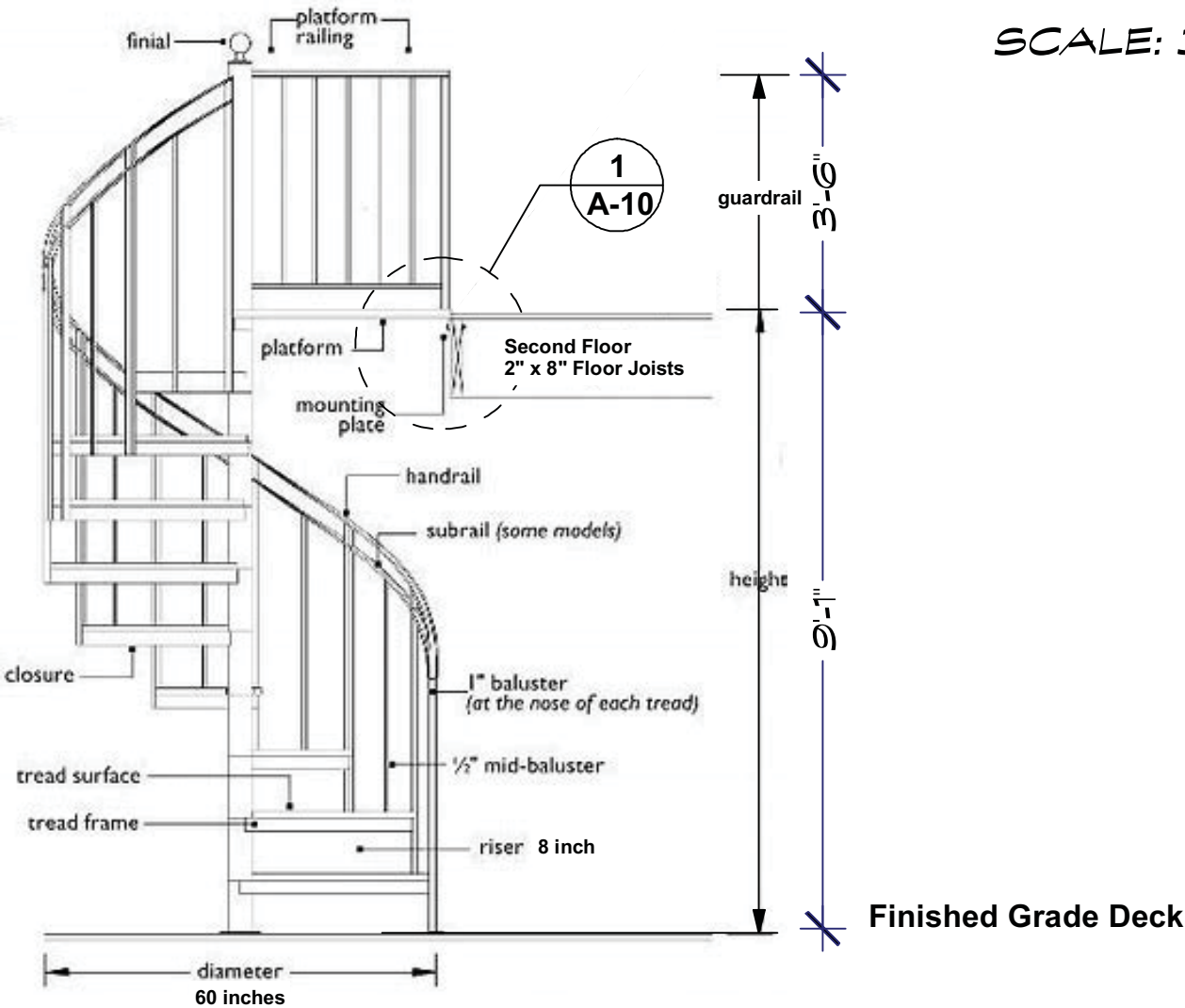
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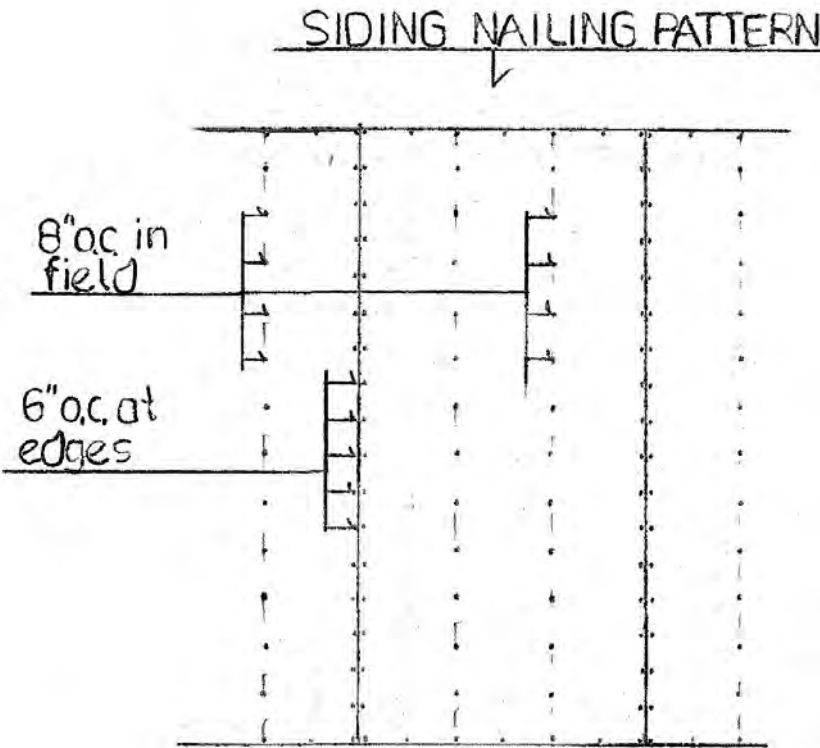
***DETAIL 1***

SCALE: 3/4" = 1'-0"



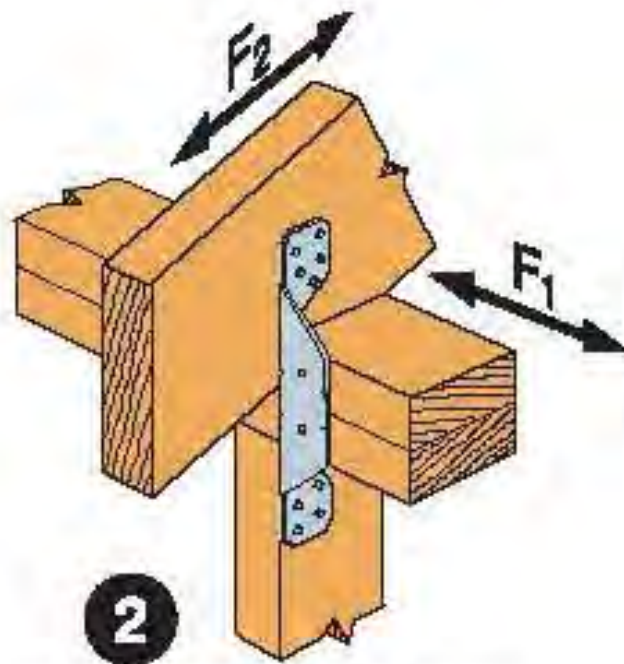
***SPIRAL STAIRCASE DETAIL***

SCALE: 3/8" = 1'-0"

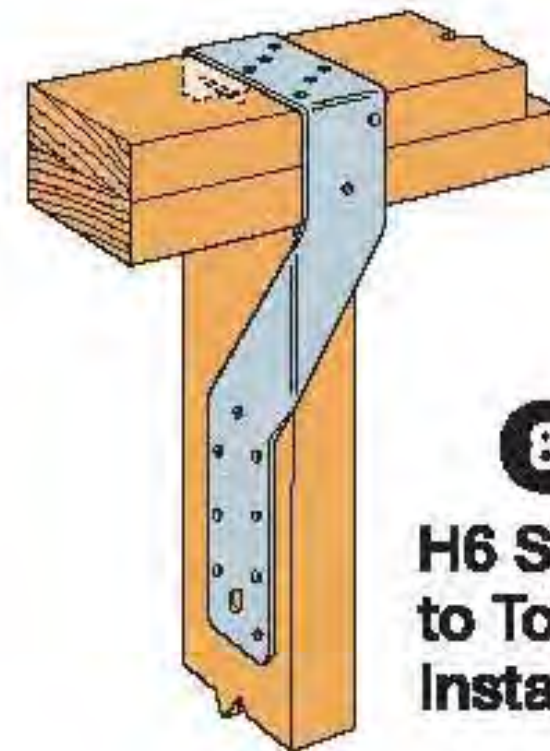


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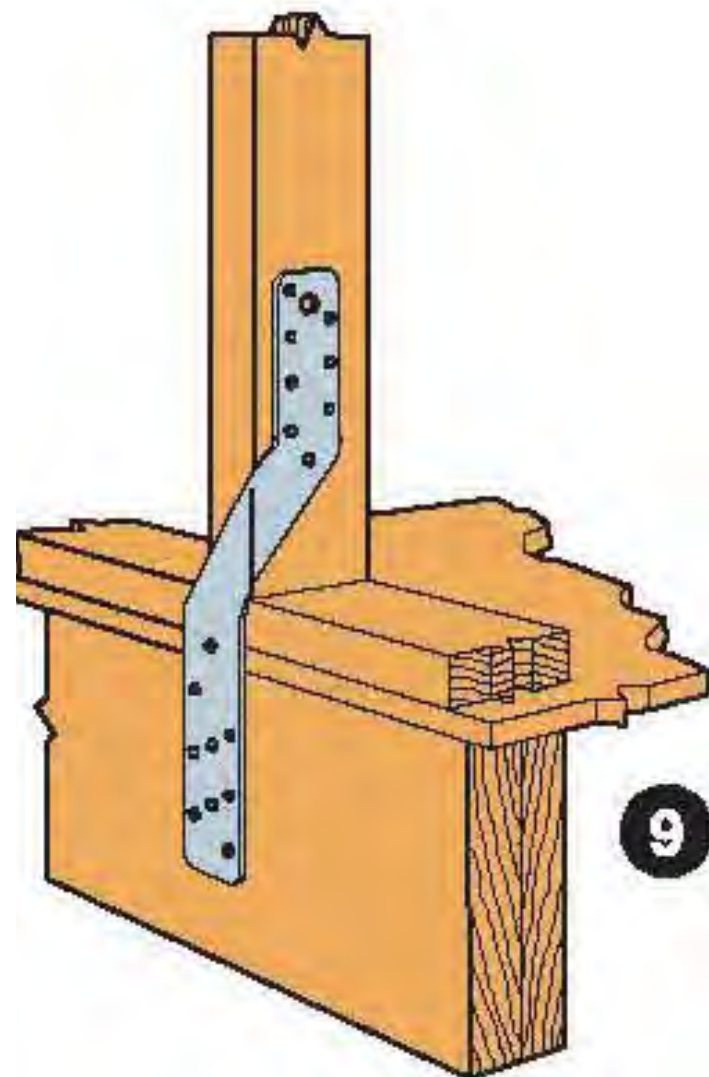


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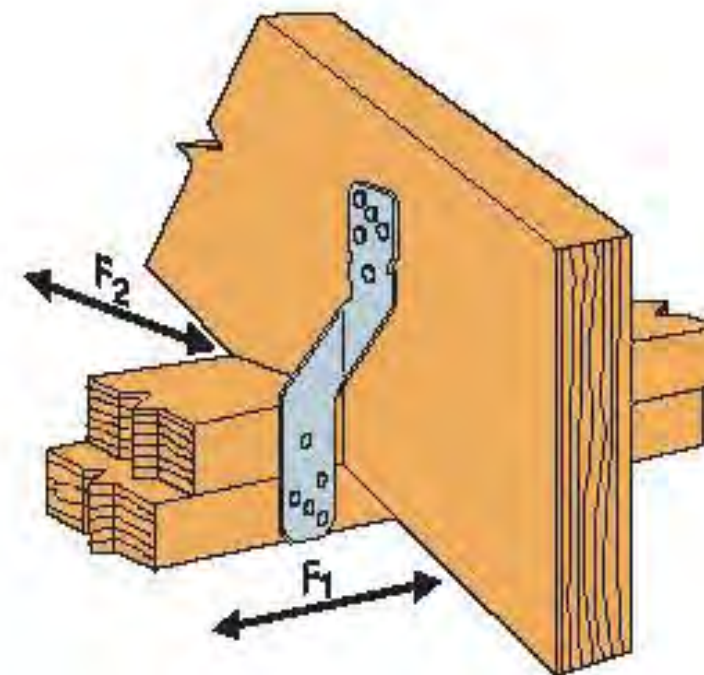
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**H6 Stud to Top Plate Installation**



9

**H6 Stud to Rim Board Installation**



11

**H8 Attaching Rafter to Double Top Plates**

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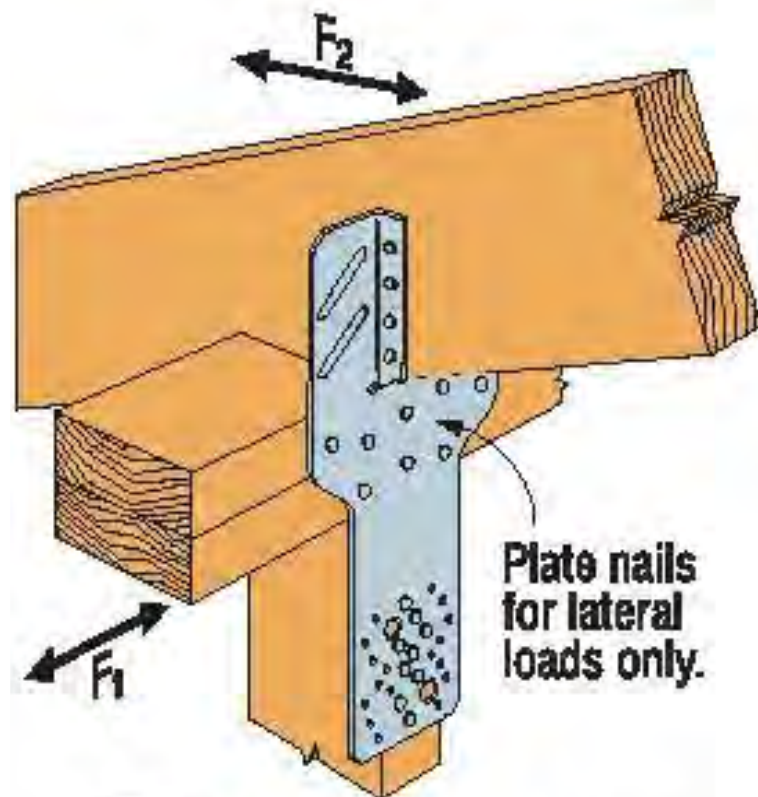
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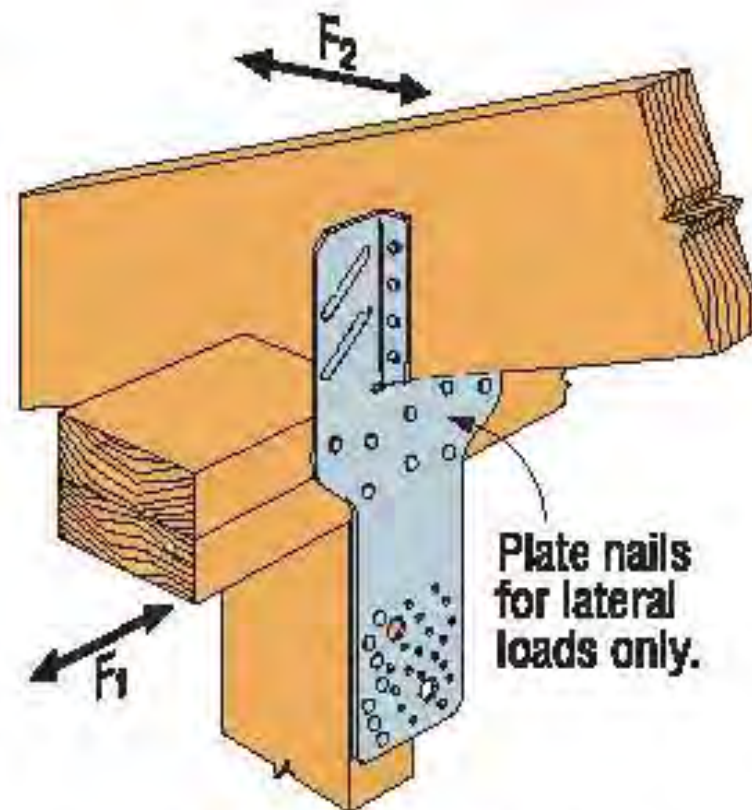
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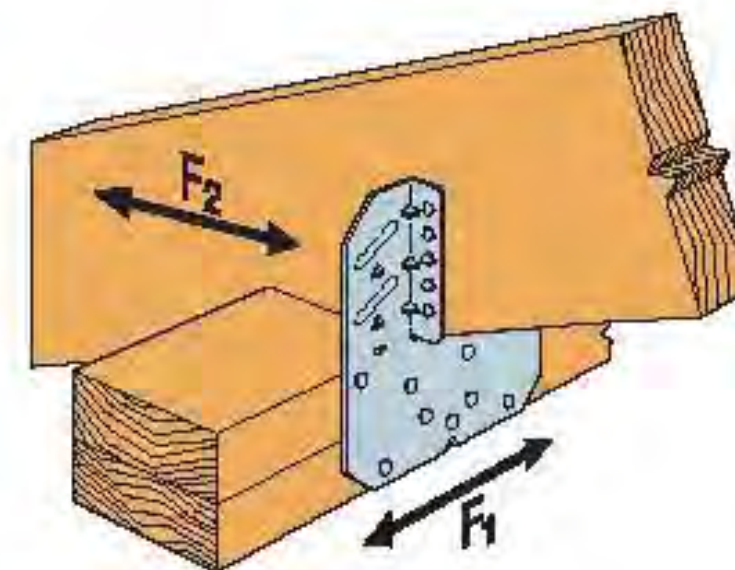




**15 H10S Installation**



**16 H10S Installation with Stud Offset**



**17 H10A Installation**

H10A optional nailing connects shear blocking to rafter. Use 0.131" x 2½" nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift load; bend one time only.

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# WIND CALCULATIONS

## MecaWind v2404

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Calculations Prepared by:  
Wanskus Architecture, P.C.  
4755 Hansard Avenue  
North Port, Florida, 34286  
Date: Sep 12, 2022  
Designer: JOHN P. WANSKUS, RA, NCARB

Calculations Prepared For:  
Client: NORTH BEACH MINI GOLF  
Project #: 22-2507.178  
Location: 120 NORTH EUCLID AVENUE, ATLANTIC  
Description: CLUBHOUSE & APT

File Location:  
C:\Users\Wanskus Architecture\WANSKUS ARCHITECTURE Dropbox\John Wanskus\PC (2)\  
Documents\MecaWind\WANSKUS ARCHITECTURE\NORTH BEACH GOLF CLUBHOUSE - APT.wnd

### Basic Wind Parameters

Wind Load Standard = ASCE 7-16 Exposure Category = D  
Wind Design Speed = 125.0 mph Risk Category = II  
Structure Type = Building Building Type = Enclosed

### General Wind Settings

Incl\_LF = Include ASD Load Factor of 0.6 in Pressures = False  
DynType = Dynamic Type of Structure = Rigid  
Zg = Altitude (Ground Elevation) above Sea Level = 0.000 ft  
Bdist = Base Elevation of Structure = 0.000 ft  
SDB = Simple Diaphragm Building = True  
Reacs = Show the Base Reactions in the output = False  
MWFRSType = MWFRS Method Selected = Ch 27 Pt 1

### Topographic Factor per Fig 26.8-1

Topo = Topographic Feature = None  
Kzt = Topographic Factor = 1.000

### Building Inputs

Roof : Building Roof Type = Monoslope W : Width Perp to Ridge = 12.000 ft  
L : Length Along Ridge = 24.000 ft Eht : Eave Height = 18.830 ft  
RE : Roof Entry Method = Slope Slope: Slope of Roof = 2.0 :12  
Theta: Roof Slope = 9.46 Deg Par : Is there a Parapet = False

### Exposure Constants per Table 26.11-1:

Alpha: Table 26.11-1 Const = 11.500 Zg: Table 26.11-1 Const = 700.000 ft  
At: Table 26.11-1 Const = 0.087 Bt: Table 26.11-1 Const = 1.070  
Am: Table 26.11-1 Const = 0.111 Bm: Table 26.11-1 Const = 0.800  
C: Table 26.11-1 Const = 0.150 Eps: Table 26.11-1 Const = 0.125

### Overhang Inputs:

Std = Overhangs on all sides are the same = True  
OHType = Type of Roof Wall Intersections = Overhang  
OH = Overhang of Roof Beyond Wall = 1.000 ft

### Main Wind Force Resisting System (MWFRS) Calculations per Ch 27 Part 1:

h = Mean Roof Height above grade = 18.830 ft  
Kh = 15 ft [4.572 m] < Z < Zg --> (2.01\*(Z/zg)^(2/Alpha)) {Table 26.10-1} = 1.072  
Kzt = Topographic Factor is 1 since no Topographic feature specified = 1.000  
Kd = Wind Directionality Factor per Table 26.6-1 = 0.85  
Zg = Elevation above Sea Level = 0.000 ft  
Ke = Ground Elevation Factor: Ke = e^-(0.0000362\*Zg) {Table 26.9-1} = 1.000  
GCPI = Ref Table 26.13-1 for Enclosed Building = +/-0.18  
RA = Roof Area = 369.02 sq ft  
LF = Load Factor based upon STRENGTH Design = 1.00  
qh = (0.00256 \* Kh \* Kzt \* Kd \* Ke \* V^2) \* LF = 36.44 psf  
qin = For Negative Internal Pressure of Enclosed Building use qh\*LF = 36.44 psf  
qip = For Positive Internal Pressure of Enclosed Building use qh\*LF = 36.44 psf

### Gust Factor Calculation:

Gust Factor Category I Rigid Structures - Simplified Method  
G1 = For Rigid Structures (Nat. Freq.>1 Hz) use 0.85 = 0.85  
Gust Factor Category II Rigid Structures - Complete Analysis  
Zm = Max(0.6 \* Ht, Zmin) = 11.298 ft  
Izm = Cc \* (33 / Zm) ^ 0.167 = 0.179  
Lzm = L \* (Zm / 33) ^ Eps = 568.492  
B = Structure Width Normal to Wind = 24.000 ft  
Q = (1 / (1 + 0.63 \* ((B + Ht) / Lzm)^0.63))^0.5 = 0.943  
G2 = 0.925\*((1+0.7\*Izm\*3.4\*Q)/(1+0.7\*3.4\*Izm)) = 0.898  
Gust Factor Used in Analysis  
G = Lessor Of G1 Or G2 = 0.850

### MWFRS Wind Normal to Ridge (Ref Fig 27.3-1)

h = Mean Roof Height Of Building = 18.830 ft  
Rht = Ridge Height Of Roof = 20.830 ft  
B = Horizontal Dimension Of Building Normal To Wind Direction = 24.000 ft  
L = Horizontal Dimension Of building Parallel To Wind Direction = 12.000 ft  
L/B = Ratio Of L/B used For Cp determination = 0.500  
h/L = Ratio Of h/L used For Cp determination = 1.569  
Slope = Slope of Roof = 9.46 Deg  
OH\_Bot\_-Y = Overhang Bottom -Y (Windward Face Only) = 0.8, 0.8  
OH\_Top = \*\*Overhang Top Coeff (0 to h/2) (0.000 ft to 9.415 ft) = -0.18, -1.149  
OH\_Top = \*\*Overhang Top Coeff (0 to h/2) (0.000 ft to 1.000 ft) = -0.18, -1.149  
OH\_Top = Overhang Top Coeff (h/2 to h) (9.415 ft to 14.000 ft) = -0.18, -0.7  
OH\_Top = Overhang Top Coeff (h/2 to h) (13.000 ft to 14.000 ft) = -0.18, -0.7  
Roof = \*\*Roof Coeff (0 to h/2) (1.000 ft to 9.415 ft) = -0.18, -1.149  
Roof = Roof Coeff (h/2 to h) (9.415 ft to 13.000 ft) = -0.18, -0.7  
\*\*Includes Reduction Factor 0.88 For roof area, applied To Cp=-1.3 For h/L>=1 & (0 To h/2)

Cp\_WW = Windward Wall Coefficient (All L/B Values) = 0.80  
Cp\_LW = Leeward Wall Coefficient using L/B = -0.50  
Cp\_SW = Side Wall Coefficient (All L/B values) = -0.70  
GCpn\_WW = Parapet Combined Net Pressure Coefficient (Windward Parapet) = 1.50  
GCpn\_LW = Parapet Combined Net Pressure Coefficient (Leeward Parapet) = -1.00

### Wall Wind Pressures based On Positive Internal Pressure (+GCPI) - Normal to Ridge

All wind pressures include a load factor of 1.0

Elev	Kz	Kzt	qz	GCPI	GCPI	Windward	Leeward	Side	Total	Minimum
ft			psf	Windward	Leeward	Press	Press	Press	Press	Pressure*
20.83	1.091	1.000	37.09	0.18	0.18	18.66	-22.05	-28.24	40.71	16.00
18.83	1.072	1.000	36.44	0.18	0.18	18.22	-22.05	-28.24	40.27	16.00
8.83	1.030	1.000	35.03	0.18	0.18	17.26	-22.05	-28.24	39.31	16.00

### Wall Wind Pressures based on Negative Internal Pressure (-GCPI) - Normal to Ridge

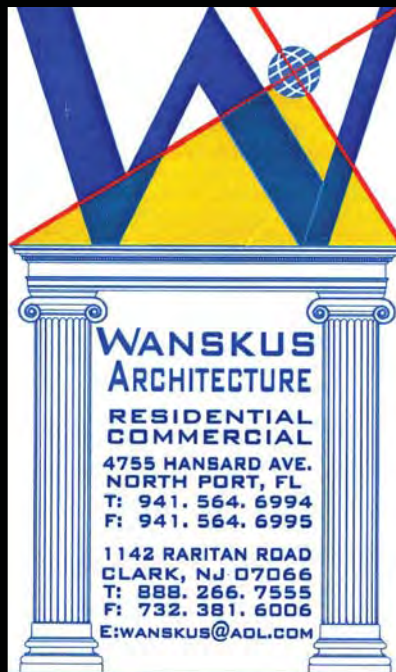
All wind pressures include a load factor of 1.0

Elev	Kz	Kzt	qz	GCPI	GCPI	Windward	Leeward	Side	Total	Minimum
ft			psf	Windward	Leeward	Press	Press	Press	Press	Pressure*
20.83	1.091	1.000	37.09	-0.18	-0.18	31.78	-8.93	-15.12	40.71	16.00
18.83	1.072	1.000	36.44	-0.18	-0.18	31.34	-8.93	-15.12	40.27	16.00
8.83	1.030	1.000	35.03	-0.18	-0.18	30.38	-8.93	-15.12	39.31	16.00

### Notes Wall Pressures:

Kz = Velocity Press Exp Coeff Kzt = Topographical Factor  
qz = 0.00256\*Kz\*Kzt\*Kd\*V^2 GCPI = Internal Press Coefficient  
Side = qh \* G \* Cp\_SW - qip \* +GCPI Windward = qz \* G \* Cp\_WW - qip \* +GCPI  
Leeward = qh \* G \* Cp\_LW - qip \* +GCPI Total = Windward Press - Leeward Press  
\* Minimum Pressure: Para 27.1.5 no less than 16.00 psf (Incl LF) applied to Walls  
+ Pressures Acting TOWARD Surface - Pressures Acting AWAY from Surface

### Roof Wind Pressures for Positive & Negative Internal Pressure (+/- GCPI) - Normal to Ridge



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# WIND CALCULATIONS

All wind pressures include a load factor of 1.0

Roof Var	Start Dist ft	End Dist ft	Cp_min	Cp_max	GCPi	Pressure Pn_min* psf	Pressure Pp_min* psf	Pressure Pn_max psf	Pressure Pp_max psf
OH_Bot_-Y	N/A	N/A	0.800	0.800	0.000	24.78	24.78	24.78	24.78
OH_Top_(-X)	0.000	9.415	-0.180	-1.149	0.000	-5.58	-5.58	-35.60	-35.60
OH_Top_(-Y)	0.000	1.000	-0.180	-1.149	0.000	-5.58	-5.58	-35.60	-35.60
OH_Top_(+X)	0.000	9.415	-0.180	-1.149	0.000	-5.58	-5.58	-35.60	-35.60
OH_Top_(-X)	9.415	14.000	-0.180	-0.700	0.000	-5.58	-5.58	-21.68	-21.68
OH_Top_(+X)	9.415	14.000	-0.180	-0.700	0.000	-5.58	-5.58	-21.68	-21.68
OH_Top_(+Y)	13.000	14.000	-0.180	-0.700	0.000	-5.58	-5.58	-21.68	-21.68
Roof (All)	1.000	9.415	-0.180	-1.149	0.180	0.98	-12.13	-29.04	-42.16
Roof (All)	9.415	13.000	-0.180	-0.700	0.180	0.98	-12.13	-15.12	-28.24

## Notes Roof Pressures:

Start Dist = Start Dist from Windward Edge      End Dist = End Dist from Windward Edge  
Cp\_Max = Largest Coefficient Magnitude      Cp\_Min = Smallest Coefficient Magnitude  
Pp\_max =  $q_h * G * Cp_{max} - q_{ip} * (+GCPi)$       Pn\_max =  $q_h * G * Cp_{max} - q_{in} * (-GCPi)$   
Pp\_min\* =  $q_h * G * Cp_{min} - q_{ip} * (+GCPi)$       Pn\_min\* =  $q_h * G * Cp_{min} - q_{in} * (-GCPi)$   
OH = Overhang      X = Dir along Ridge      Y = Dir Perpendicular to Ridge      Z = Vertical  
\* The smaller uplift pressures due to Cp\_Min can become critical when wind is combined with roof live load or snow load; load combinations are given in ASCE 7  
+ Pressures Acting TOWARD Surface      - Pressures Acting AWAY from Surface

## MWFRS Wind Normal to Eave (Ref Fig 27.3-1)

h	= Mean Roof Height Of Building	= 18.830 ft
RHt	= Ridge Height Of Roof	= 20.830 ft
B	= Horizontal Dimension Of Building Normal To Wind Direction	= 24.000 ft
L	= Horizontal Dimension Of building Parallel To Wind Direction	= 12.000 ft
L/B	= Ratio Of L/B used For Cp determination	= 0.500
h/L	= Ratio Of h/L used For Cp determination	= 1.569
Slope	= Slope of Roof	= 9.46 Deg
OH_Bot_+Y	= Overhang Bottom +Y (Windward Face Only)	= 0.8, 0.8
OH_Top	= **Overhang Top Coeff (0 to h/2) (0.000 ft to 9.415 ft)	= -0.18, -1.149
OH_Top	= **Overhang Top Coeff (0 to h/2) (0.000 ft to 1.000 ft)	= -0.18, -1.149
OH_Top	= Overhang Top Coeff (h/2 to h) (9.415 ft to 14.000 ft)	= -0.18, -0.7
OH_Top	= Overhang Top Coeff (h/2 to h) (13.000 ft to 14.000 ft)	= -0.18, -0.7
Roof	= **Roof Coeff (0 to h/2) (1.000 ft to 9.415 ft)	= -0.18, -1.149
Roof	= Roof Coeff (h/2 to h) (9.415 ft to 13.000 ft)	= -0.18, -0.7
**Includes Reduction Factor 0.88 For roof area, applied To Cp=-1.3 For h/L>=1 & (0 To h/2)		

Cp_WW	= Windward Wall Coefficient (All L/B Values)	= 0.80
Cp_LW	= Leward Wall Coefficient using L/B	= -0.50
Cp_SW	= Side Wall Coefficient (All L/B values)	= -0.70
GCpn_WW	= Parapet Combined Net Pressure Coefficient (Windward Parapet)	= 1.50
GCpn_LW	= Parapet Combined Net Pressure Coefficient (Leeward Parapet)	= -1.00

## Wall Wind Pressures based On Positive Internal Pressure (+GCPi) - Normal to Eave All wind pressures include a load factor of 1.0

Elev	Kz	Kzt	qz	GCPi	GCPi	Windward	Leeward	Side	Total	Minimum
ft			psf	Windward	Leeward	Press	Press	Press	Press	Pressure*
18.83	1.072	1.000	36.44	0.18	0.18	18.22	-22.05	-28.24	40.27	16.00
8.83	1.030	1.000	35.03	0.18	0.18	17.26	-22.05	-28.24	39.31	16.00

## Wall Wind Pressures based on Negative Internal Pressure (-GCPi) - Normal to Eave All wind pressures include a load factor of 1.0

Elev	Kz	Kzt	qz	GCPi	GCPi	Windward	Leeward	Side	Total	Minimum
ft			psf	Windward	Leeward	Press	Press	Press	Press	Pressure*

18.83	1.072	1.000	36.44	-0.18	-0.18	31.34	-8.93	-15.12	40.27	16.00
8.83	1.030	1.000	35.03	-0.18	-0.18	30.38	-8.93	-15.12	39.31	16.00

## Notes Wall Pressures:

Kz = Velocity Press Exp Coeff      Kzt = Topographical Factor  
qz =  $0.00256 * Kz * Kzt * Kd * V^2$       GCPi = Internal Press Coefficient  
Side =  $q_h * G * Cp_{SW} - q_{ip} * +GCPi$       Windward =  $q_z * G * Cp_{WW} - q_{ip} * +GCPi$   
Leeward =  $q_h * G * Cp_{LW} - q_{ip} * +GCPi$       Total = Windward Press - Leeward Press  
\* Minimum Pressure: Para 27.1.5 no less than 16.00 psf (Incl LF) applied to Walls  
+ Pressures Acting TOWARD Surface      - Pressures Acting AWAY from Surface

## Roof Wind Pressures for Positive & Negative Internal Pressure (+/- GCPi) - Normal to Eave All wind pressures include a load factor of 1.0

Roof Var	Start Dist ft	End Dist ft	Cp_min	Cp_max	GCPi	Pressure Pn_min* psf	Pressure Pp_min* psf	Pressure Pn_max psf	Pressure Pp_max psf
OH_Bot_+Y	N/A	N/A	0.800	0.800	0.000	24.78	24.78	24.78	24.78
OH_Top_(-X)	0.000	9.415	-0.180	-1.149	0.000	-5.58	-5.58	-35.60	-35.60
OH_Top_(+Y)	0.000	1.000	-0.180	-1.149	0.000	-5.58	-5.58	-35.60	-35.60
OH_Top_(+X)	0.000	9.415	-0.180	-1.149	0.000	-5.58	-5.58	-35.60	-35.60
OH_Top_(-X)	9.415	14.000	-0.180	-0.700	0.000	-5.58	-5.58	-21.68	-21.68
OH_Top_(+X)	9.415	14.000	-0.180	-0.700	0.000	-5.58	-5.58	-21.68	-21.68
OH_Top_(-Y)	13.000	14.000	-0.180	-0.700	0.000	-5.58	-5.58	-21.68	-21.68
Roof (All)	1.000	9.415	-0.180	-1.149	0.180	0.98	-12.13	-29.04	-42.16
Roof (All)	9.415	13.000	-0.180	-0.700	0.180	0.98	-12.13	-15.12	-28.24

## Notes Roof Pressures:

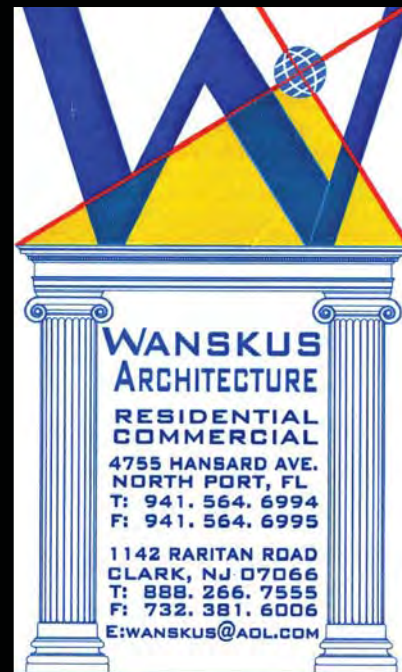
Start Dist = Start Dist from Windward Edge      End Dist = End Dist from Windward Edge  
Cp\_Max = Largest Coefficient Magnitude      Cp\_Min = Smallest Coefficient Magnitude  
Pp\_max =  $q_h * G * Cp_{max} - q_{ip} * (+GCPi)$       Pn\_max =  $q_h * G * Cp_{max} - q_{in} * (-GCPi)$   
Pp\_min\* =  $q_h * G * Cp_{min} - q_{ip} * (+GCPi)$       Pn\_min\* =  $q_h * G * Cp_{min} - q_{in} * (-GCPi)$   
OH = Overhang      X = Dir along Ridge      Y = Dir Perpendicular to Ridge      Z = Vertical  
\* The smaller uplift pressures due to Cp\_Min can become critical when wind is combined with roof live load or snow load; load combinations are given in ASCE 7  
+ Pressures Acting TOWARD Surface      - Pressures Acting AWAY from Surface

## MWFRS Wind Parallel to Ridge (Ref Fig 27.3-1)

h	= Mean Roof Height Of Building	= 18.830 ft
RHt	= Ridge Height Of Roof	= 20.830 ft
B	= Horizontal Dimension Of Building Normal To Wind Direction	= 12.000 ft
L	= Horizontal Dimension Of building Parallel To Wind Direction	= 24.000 ft
L/B	= Ratio Of L/B used For Cp determination	= 2.000
h/L	= Ratio Of h/L used For Cp determination	= 0.785
Slope	= Slope of Roof	= 9.46 Deg
OH_Bot_-X	= Overhang Bottom -X (Windward Face Only)	= 0.8, 0.8
OH_Top	= **Overhang Top Coeff (0 to h/2) (0.000 ft to 1.000 ft)	= -0.18, -1.042
OH_Top	= **Overhang Top Coeff (0 to h/2) (1.000 ft to 9.415 ft)	= -0.18, -1.042
OH_Top	= Overhang Top Coeff (h/2 to h) (9.415 ft to 18.830 ft)	= -0.18, -0.786
OH_Top	= Overhang Top Coeff (h to 2h) (18.830 ft to 25.000 ft)	= -0.18, -0.614
OH_Top	= Overhang Top Coeff (h to 2h) (25.000 ft to 26.000 ft)	= -0.18, -0.614
Roof	= **Roof Coeff (0 to h/2) (1.000 ft to 9.415 ft)	= -0.18, -1.042
Roof	= Roof Coeff (h/2 to h) (9.415 ft to 18.830 ft)	= -0.18, -0.786
Roof	= Roof Coeff (h to 2h) (18.830 ft to 25.000 ft)	= -0.18, -0.614
**Includes Reduction Factor 0.88 For roof area, applied To Cp=-1.3 For h/L>=1 & (0 To h/2)		

Cp_WW	= Windward Wall Coefficient (All L/B Values)	= 0.80
Cp_LW	= Leward Wall Coefficient using L/B	= -0.30
Cp_SW	= Side Wall Coefficient (All L/B values)	= -0.70
GCpn_WW	= Parapet Combined Net Pressure Coefficient (Windward Parapet)	= 1.50
GCpn_LW	= Parapet Combined Net Pressure Coefficient (Leeward Parapet)	= -1.00

## Wall Wind Pressures based On Positive Internal Pressure (+GCPi) - Parallel to Ridge All wind pressures include a load factor of 1.0



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## DRAWING TITLE

## WIND CALCS

NO.	REV.	DATE
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DRAWN BY: JPW  
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PROJECT # 22-2507.178  
DATE: 25 JULY 2022

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16 OF 20

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WIND CALCULATIONS

Elev	Kz	Kzt	qz	GCPi	GCPi	Windward	Leeward	Side	Total	Minimum
ft			psf	Windward	Leeward	Press	Press	Press	Press	Pressure*
20.83	1.091	1.000	37.09	0.18	0.18	18.66	-15.85	-28.24	34.51	16.00
18.83	1.072	1.000	36.44	0.18	0.18	18.22	-15.85	-28.24	34.07	16.00
8.83	1.030	1.000	35.03	0.18	0.18	17.26	-15.85	-28.24	33.11	16.00

Wall Wind Pressures based on Negative Internal Pressure (-GCPi) - Parallel to Ridge  
All wind pressures include a load factor of 1.0

Elev	Kz	Kzt	qz	GCPi	GCPi	Windward	Leeward	Side	Total	Minimum
ft			psf	Windward	Leeward	Press	Press	Press	Press	Pressure*
20.83	1.091	1.000	37.09	-0.18	-0.18	31.78	-2.73	-15.12	34.51	16.00
18.83	1.072	1.000	36.44	-0.18	-0.18	31.34	-2.73	-15.12	34.07	16.00
8.83	1.030	1.000	35.03	-0.18	-0.18	30.38	-2.73	-15.12	33.11	16.00

Notes Wall Pressures:

Kz = Velocity Press Exp Coeff  
qz = 0.00256\*Kz\*Kzt\*Kd\*V^2  
Side = qh \* G \* Cp\_SW - qip \* +GCPi  
Leeward = qh \* G \* Cp\_LW - qip \* +GCPi  
\* Minimum Pressure: Para 27.1.5 no less than 16.00 psf (Incl LF) applied to Walls  
+ Pressures Acting TOWARD Surface

Kzt = Topographical Factor  
GCPi = Internal Press Coefficient  
Windward = qz \* G \* Cp\_WW - qip \* +GCPi  
Total = Windward Press - Leeward Press  
- Pressures Acting AWAY from Surface

Roof Wind Pressures for Positive & Negative Internal Pressure (+/- GCPi) - Parallel to Ridge  
All wind pressures include a load factor of 1.0

Roof Var	Start Dist	End Dist	Cp_min	Cp_max	GCPi	Pressure Pn_min*	Pressure Pp_min*	Pressure Pn_max	Pressure Pp_max
ft	ft	ft				psf	psf	psf	psf
OH_Bot_-X	N/A	N/A	0.800	0.800	0.000	24.78	24.78	24.78	24.78
OH_Top_(-X)	0.000	1.000	-0.180	-1.042	0.000	-5.58	-5.58	-32.27	-32.27
OH_Top_(-Y)	1.000	9.415	-0.180	-1.042	0.000	-5.58	-5.58	-32.27	-32.27
OH_Top_(+Y)	1.000	9.415	-0.180	-1.042	0.000	-5.58	-5.58	-32.27	-32.27
OH_Top_(-Y)	9.415	18.830	-0.180	-0.786	0.000	-5.58	-5.58	-24.35	-24.35
OH_Top_(+Y)	9.415	18.830	-0.180	-0.786	0.000	-5.58	-5.58	-24.35	-24.35
OH_Top_(-Y)	18.830	25.000	-0.180	-0.614	0.000	-5.58	-5.58	-19.01	-19.01
OH_Top_(+Y)	18.830	25.000	-0.180	-0.614	0.000	-5.58	-5.58	-19.01	-19.01
OH_Top_(+X)	25.000	26.000	-0.180	-0.614	0.000	-5.58	-5.58	-19.01	-19.01
Roof (All)	1.000	9.415	-0.180	-1.042	0.180	0.98	-12.13	-25.71	-38.83
Roof (All)	9.415	18.830	-0.180	-0.786	0.180	0.98	-12.13	-17.79	-30.91
Roof (All)	18.830	25.000	-0.180	-0.614	0.180	0.98	-12.13	-12.45	-25.57

Notes Roof Pressures:

Start Dist = Start Dist from Windward Edge  
Cp\_Max = Largest Coefficient Magnitude  
Pp\_max = qh\*G\*Cp\_max - qip\*(+GCPi)  
Pp\_min\* = qh\*G\*Cp\_min - qip\*(+GCPi)  
OH = Overhang X = Dir along Ridge Y = Dir Perpendicular to Ridge Z = Vertical  
\* The smaller uplift pressures due to Cp\_Min can become critical when wind is combined with roof live load or snow load; load combinations are given in ASCE 7  
+ Pressures Acting TOWARD Surface

End Dist = End Dist from Windward Edge  
Cp\_Min = Smallest Coefficient Magnitude  
Pn\_max = qh\*G\*Cp\_max - qin\*(-GCPi)  
Pn\_min\* = qh\*G\*Cp\_min - qin\*(-GCPi)  
- Pressures Acting AWAY from Surface

Components and Cladding (C&C) Zone Summary per Ch 30 Pt 4:

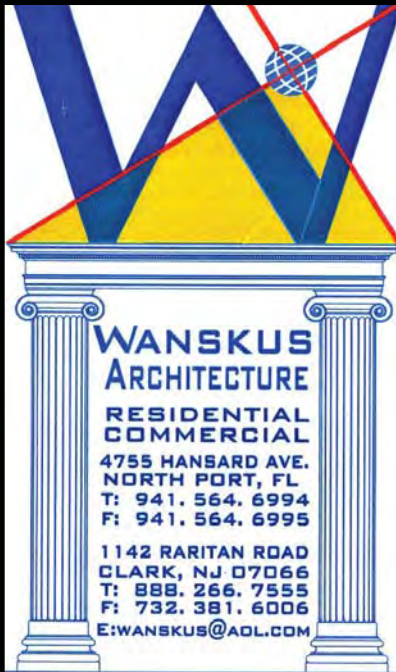
h = Mean Roof Height = 18.830 ft  
L = Load Factor based upon STRENGTH Design = 1.00  
Z = Topographic Factor is 1 since no Topographic feature specified = 1.000  
F = Adjustment factor per Table 30.6-2 to Fig 30.4-1 pressures = 1.528  
Slope = Roof Slope = 9.46 Deg

LHD = Least Horizontal Dimension: Min(B, L) = 12.000 ft  
a1 = Min(0.1 \* LHD, 0.4 \* h) = 1.200 ft  
a = Max(a1, 0.04 \* LHD, 3 ft [0.9 m]) = 3.000 ft  
2a = Parameter used to define zone width: 2\*a = 6.000 ft  
EAF = Adjustment factor per Table 30.6-2 to Fig 30.4-1 pressures = 1.528

Wind Pressure Summary for C&C Zones based Upon Areas Ch 30 Pt 4  
All wind pressures include a load factor of 1.0

Zone	Table	A <= 10.00 sq ft	A = 20.00 sq ft	A = 50.00 sq ft	A > 100.00 sq ft
1	30.4-1	26.06 -79.56	23.46 -79.56	20.10 -48.38	20.10 -48.38
1_OH	30.4-1	16.00 -91.17	16.00 -91.17	16.00 -78.87	16.00 -78.87
2e	30.4-1	26.06 -79.56	23.46 -79.56	20.10 -48.38	20.10 -48.38
2e_OH	30.4-1	16.00 -91.17	16.00 -91.17	16.00 -78.87	16.00 -78.87
2n	30.4-1	26.06 -115.93	23.46 -100.27	20.10 -79.56	20.10 -79.56
2n_OH	30.4-1	16.00 -127.63	16.00 -111.12	16.00 -89.41	16.00 -89.41
2r	30.4-1	26.06 -115.93	23.46 -100.27	20.10 -79.56	20.10 -79.56
2r_OH	30.4-1	16.00 -127.63	16.00 -111.12	16.00 -89.41	16.00 -89.41
3e	30.4-1	26.06 -115.93	23.46 -100.27	20.10 -79.56	20.10 -79.56
3e_OH	30.4-1	16.00 -149.48	16.00 -129.08	16.00 -102.10	16.00 -102.10
3r	30.4-1	26.06 -137.87	23.46 -118.07	20.10 -92.01	20.10 -92.01
3r_OH	30.4-1	16.00 -175.08	16.00 -147.65	16.00 -111.27	16.00 -111.27
4	30.4-1	43.03 -46.69	41.04 -44.71	38.52 -42.19	38.52 -42.19
5	30.4-1	43.03 -57.62	41.04 -53.80	38.52 -48.60	38.52 -48.60

\* A is effective wind area for C&C: Span Length \* Effective Width  
\* Effective width need not be less than 1/3 of the span length  
\* Maximum and minimum values of pressure shown.  
\* + Pressures acting toward surface, - Pressures acting away from surface  
\* \_OH represents an Overhang in the zone specified  
\* Overhangs follow Sec 30.6.1.3, Zones as shown in Table 30.6-2 with pressures from Fig 30.4-1  
\* Per Para 30.2.2 the Minimum Pressure for C&C is 16.00 psf [0.766 kPa] {Includes LF}  
\* Interpolation can be used for values of A that are between those values shown.



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WIND  
CALCS

NO. REV. DATE

OWNER / LOCATION  
NORTH BEACH MINI GOLF  
120 EUCLID AVENUE  
ATLANTIC CITY, NJ

DRAWN BY: JPW  
CHECKED BY: JPW  
PROJECT # 22-2507.178  
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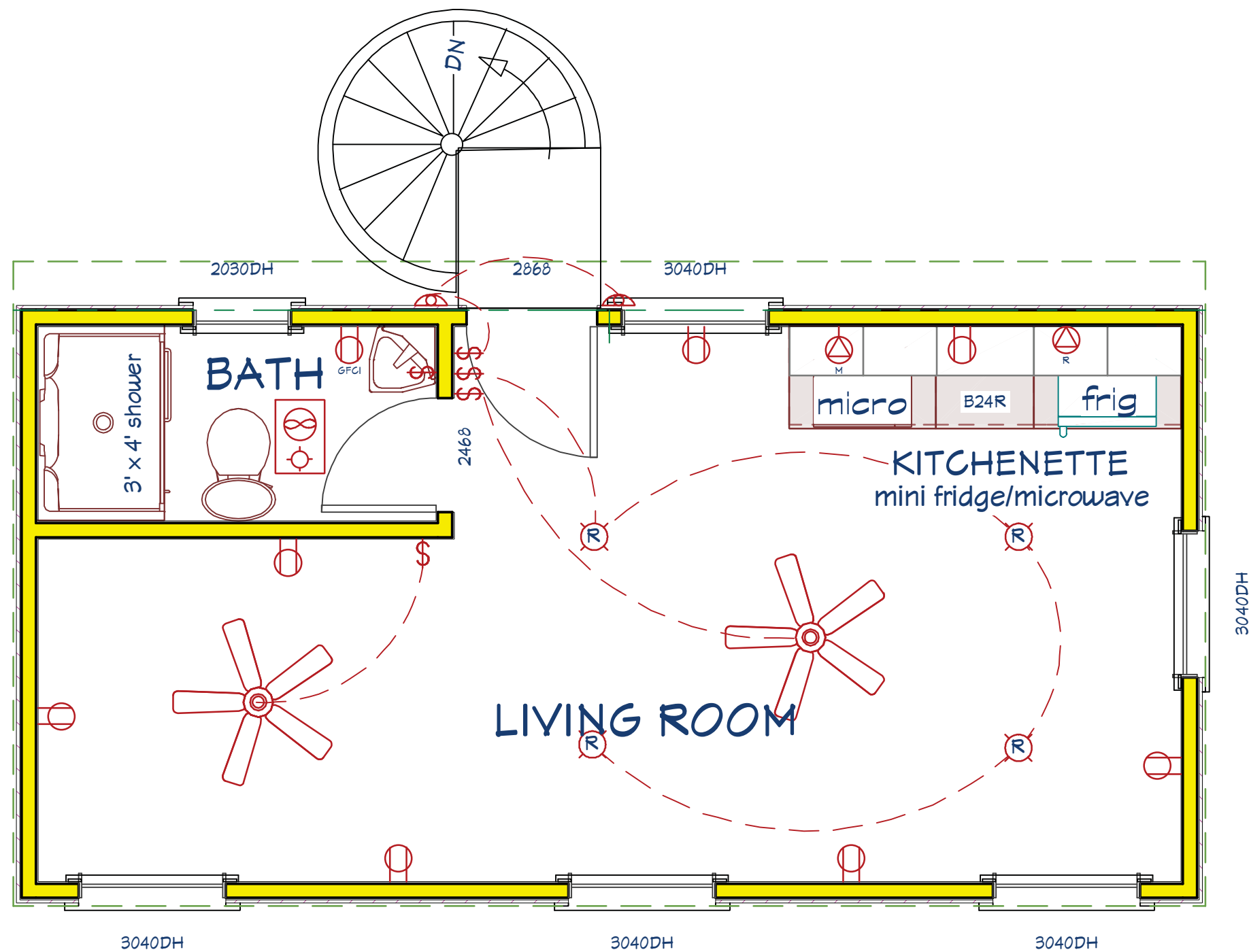
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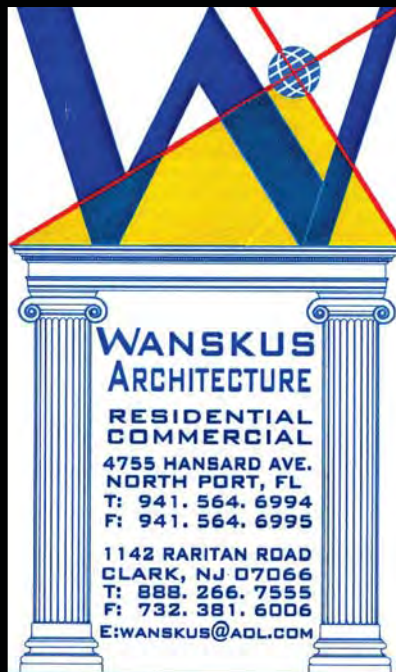
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# SECOND FLOOR ELECTRICAL PLAN

SCALE: 3/8" = 1'-0"



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- ☐ IL ~ #001-014842
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- ☒ NJ ~ #21A101169200
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- ☐ WY ~ #C-3265



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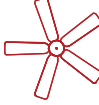


















DRAWING TITLE  
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DRAWN BY: JPW CHECKED BY: JPW PROJECT # 22-2507.178 DATE: 25 JULY 2022		

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ELECTRICAL - DATA - AUDIO LEGEND	
SYMBOL	DESCRIPTION
	Ceiling Fan
	Ventilation Fans: Ceiling Mounted, Wall Mounted
	Ceiling Mounted Light Fixtures: Surface/Pendant, Recessed, Heat Lamp, Low Voltage
	Wall Mounted Light Fixtures: Flush Mounted, Wall Sconce
	Chandelier Light Fixture
	Fluorescent Light Fixture
	240V Receptacle
	110V Receptacles: Duplex, Weather Proof, GFCI
	Switches: Single Pole, Weather Proof, 3-Way, 4-Way
	Switches: Dimmer, Timer
	Audio Video: Control Panel, Switch
	Speakers: Ceiling Mounted, Wall Mounted
	Wall Jacks: CAT5, CAT5 + TV, TV/Cable
	Telephone Jack
	Intercom
	Thermostat
	Door Chime, Door Bell Button
	Smoke Detectors: Ceiling Mounted, Wall Mounted
	Electrical Breaker Panel

**ELECTRICAL LEGEND**  
SCALE: N.T.S.

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□ NY ~ #03-034472  
□ NC ~ #014537  
□ SC ~ #AR9931  
□ TX ~ #21153  
□ WY ~ #C-3265



STATE OF NEW JERSEY  
JOHN PETER WANSKUS  
11692  
10/29/22  
REGISTERED ARCHITECT

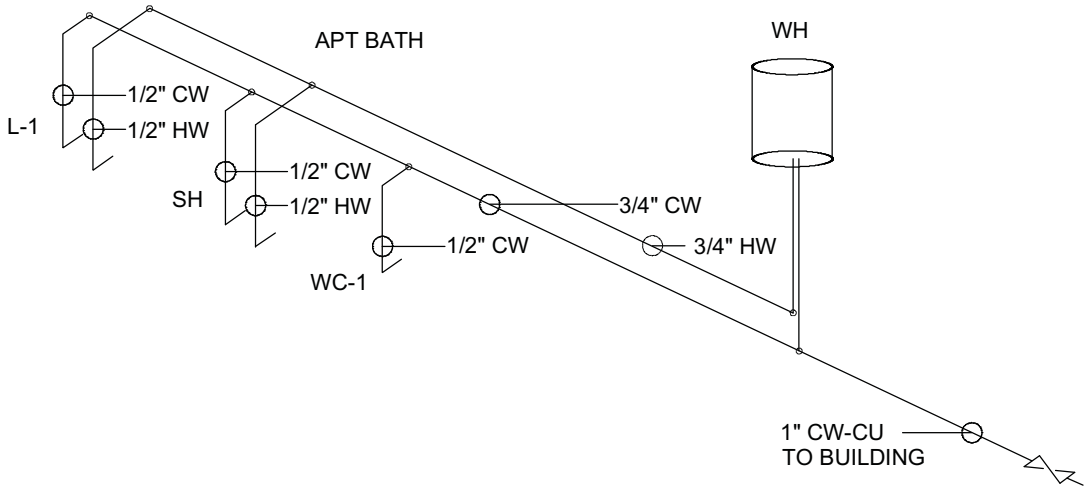
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**DRAWING TITLE**  
***ELECTRICAL  
NOTES &  
SCHEDULE***

NO.	REV.	DATE
<b>OWNER / LOCATION</b> NORTH BEACH MINI GOLF 120 EUCLID AVENUE ATLANTIC CITY, NJ		
<b>DRAWN BY:</b> JPW <b>CHECKED BY:</b> JPW <b>PROJECT #</b> 22-2507.178 <b>DATE:</b> 25 JULY 2022		
<b>SHEET NO.</b> <b>E-2</b> 19 OF 20		

FIXTURE CONNECTION SCHEDULE

SYM.	DESCRIPTION	C.W.	H.W.	WASTE	VENT
WC-1	WATER CLOSET	1/2"	—	3"	3"
WC-2	WATER CLOSET (A.D.A.)	1/2"	—	3"	3"
L-1	LAVATORY - BATH (COUNTER/PEDESTAL)	3/8"	3/8"	2"	2"
L-2	LAVATORY - BATH (ADA WALL HUNG)	—	—	—	—
SS	MOP SINK	—	—	—	—
S-1	HAND SINK	—	—	—	—
S-2	BAR SINK	—	—	—	—
S-3	THREE COMPARTMENT SINK	—	—	—	—
UR	URINAL	—	—	—	—
FD-1	FLOOR DRAIN	—	—	3"	3"
GT	GREASE TRAP	—	—	—	—
CF	COLD FOOD TABLE	—	—	—	—
ICE	REFRIGERATOR ICE MAKER	—	—	—	—
ICO	INTERIOR CLEAN-OUT	—	—	SEE NOTE	—
SH	SHOWER	1/2"	1/2"	3"	3"
DW	DISHWASHER	—	—	—	—
TUB	BATH TUB	—	—	—	—
HT-1	WHIRLPOOL TUB	—	—	—	—
WH	WATER HEATER - 20 GAL	3/4"	3/4"	2"	2"
SG	SODA GUN	—	—	—	—
BS	BLENDER STATION	—	—	—	—
DB	DRAIN BOARD	—	—	—	—
WB	WET BAR SINK	—	—	—	—



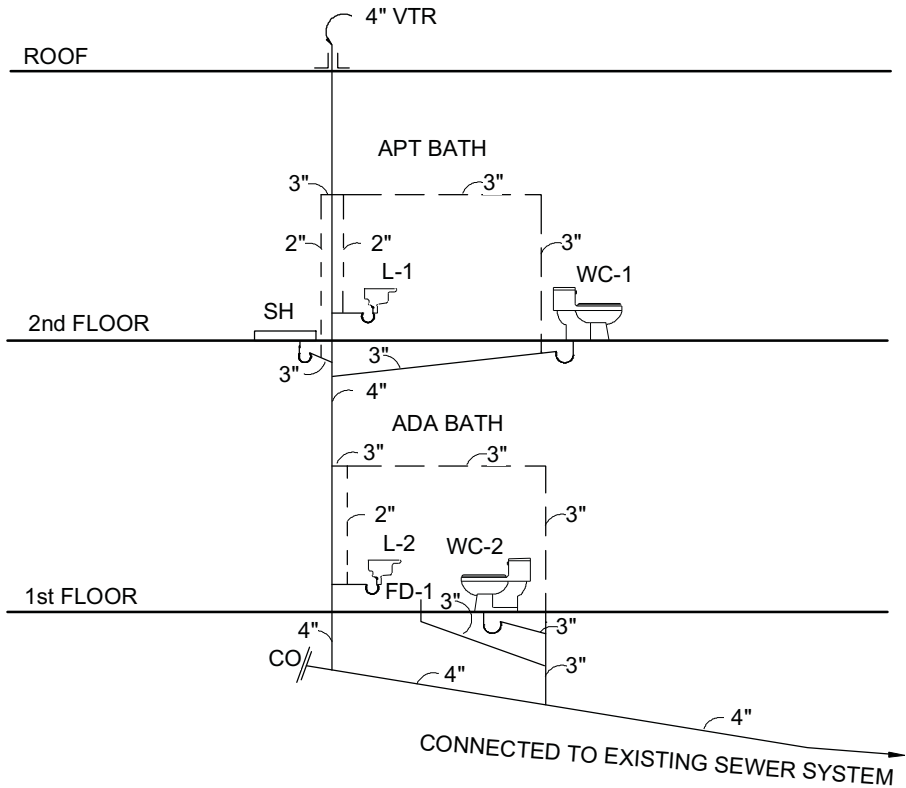
WATER RISER DIAGRAM  
DIAGRAMMATIC

R.P.P BACK  
FLOW

NOTE  
HOT WATER PIPING  
REQUIRED INSULATION

PLUMBING SPECIFICATIONS

1. FURNISH, INSTALL, PROVIDE AND MAKE OPERATIVE ALL EQUIPMENT, MATERIALS, SUPERVISION, LABOR AND ANY AND ALL ITEMS NECESSARY FOR THE PROPER INSTALLATION OF A CORRECTLY FUNCTIONING PLUMBING SYSTEM AS SHOWN ON THE DRAWINGS AND SPECIFIED HEREIN.
2. EQUIPMENT SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. EQUALS SHALL BE ACCEPTED FOR EQUIPMENT UNLESS OTHERWISE NOTED.
3. ORDINANCES, PERMITS AND CODES, THE WORKMANSHIP AND MATERIALS COVERED BY THESE SPECIFICATIONS SHALL CONFORM TO ALL REGULATIONS OF ALL THE AUTHORITIES HAVING JURISDICTION.
4. FURNISH, INSTALL, PROVIDE AND MAKE OPERATIVE A HOT WATER HEATER - 30 GALLON (FOR ALL SINKS AND EQUIPMENT).
5. THE LOCATION OF PIPING AND EQUIPMENT, AS SHOWN ON THE DRAWINGS, IS DIAGRAMMATIC AND SCHEMATIC AND IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO MAKE HIS OWN WORKING LAYOUT TO ELIMINATE, IF ANY, ALL STRUCTURAL AND ARCHITECTURAL CONFLICTS IN THE BUILDING.
6. VERIFY ALL MEASUREMENTS AT THE SITE AND COORDINATE ALL WORK SO THAT IT DOES NOT INTERFERE WITH THE WORK OF THE OTHER TRADES.
7. INSULATION: ALL INSULATION, INCLUDING JACKET, OR FACING AND ADHESIVE USED TO ADHERE FACING OR JACKET TO THE INSULATION SHALL HAVE A COMPOSITE FIRE AND SMOKE HAZARD RATING TESTED BY THE PROCEDURE RECOMMENDED BY ASTM E-84, NFPA 225 OR U.L. 723, NOT EXCEEDING: FLAME SPREAD 25, SMOKE DEVELOPED 50. ALL INSULATION ACCESSORIES SHALL ALSO HAVE THE RATING LISTED ABOVE.
8. DOMESTIC WATER PIPES SHALL BE INSULATED WITH ONE (1) INCH THICK PREFORMED HIGH DENSITY FIBERGLAS WITH FACTORY APPLIED VAPOR BARRIER AND SELF-SEALING LAP, SUCH AS OWENS CORNING 25 ASJ.
9. DOMESTIC WATER PIPES SHALL BE PVC ABOVE GROUND/SLAB AND BELOW GRADE/SLAB.
10. SANITARY WASTE AND VENT PIPE SHALL BE PVC WITH NO-HUB FITTINGS OR SCHEDULE 40 PVC.
11. NATURAL GAS PIPE SHALL BE SCHEDULE 40 BLACK STEEL WITH THREADED FITTINGS. INSTALL PRESSURE REDUCING VALVES WHERE REQUIRED. SYSTEM TO BE DESIGNED FOR 2.0 PSIG PRESSURE. INSTALL IN ACCORDANCE WITH NFPA 54.
12. CLEAN-OUTS INSTALLED TO BE THE SAME SIZE AS THE LINE SERVED.



VENT/WASTE RISER DIAGRAM  
DIAGRAMMATIC

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DRAWN BY: JPW CHECKED BY: JPW PROJECT # 22-2507.178 DATE: 25 JULY 2022		

SHEET NO.  
**P-1**  
20 OF 20